

# STILLAGUAMISH TRIBAL COURTHOUSE

17014 59TH AVENUE NE, ARLINGTON, WASHINGTON



## PROJECT CRITERIA

### SCOPE OF WORK

EXISTING BUILDING WILL BE MODIFIED TO ACCOMMODATE A NEW TRIBAL COURTROOM AND ASSOCIATED SUPPORT FACILITIES

TAX ACCOUNT NOS.: 31052700202100

### LEGAL DESCRIPTION

SEC 27 TWP 31 RGE 05 QUARTER NW LOT 2 CITY OF ARLINGTON, WA 98201 BEING A PTN OF NE1/4 NW1/4.

### CODES

2015 INTERNATIONAL BUILDING CODE (IBC)  
2015 INTERNATIONAL MECHANICAL CODE (IMC)  
2015 INTERNATIONAL FIRE CODE (IFC)  
2015 UNIFORM PLUMBING CODE (UPC)

### BUILDING CRITERIA

ZONING: GC (GENERAL COMMERCIAL)  
CONSTRUCTION TYPE: V-B (FULLY SPRINKLERED & MONITORED)  
IBC OCCUPANCY: B

### UTILITY DISTRICTS

POWER: SNOHOMISH COUNTY PUD  
WATER: CITY OF ARLINGTON  
SEWER: CITY OF ARLINGTON  
TELEPHONE: VERIZON  
GAS: CASCADE NATURAL GAS

## PROJECT CONTACTS

### LEGAL OWNER

STILLAGUAMISH TRIBE OF INDIANS  
3922 236TH ST NE  
ARLINGTON, WA 98223  
ATTN: JEREMY SMITH

### ARCHITECT

2812 ARCHITECTURE  
2812 COLBY AVE  
EVERETT, WA 98201  
425-252-2153

### STRUCTURAL ENGINEER

QUANTUM CONSULTING ENGINEERING LLC  
1511 THIRD AVE, STE 323  
SEATTLE, WASHINGTON 98101

### MEP

RENSCH ENGINEERING  
111 AVE C, SUITE 104  
SNOHOMISH, WA 98223  
PHN: 360-863-6677

## INDEX OF DRAWINGS

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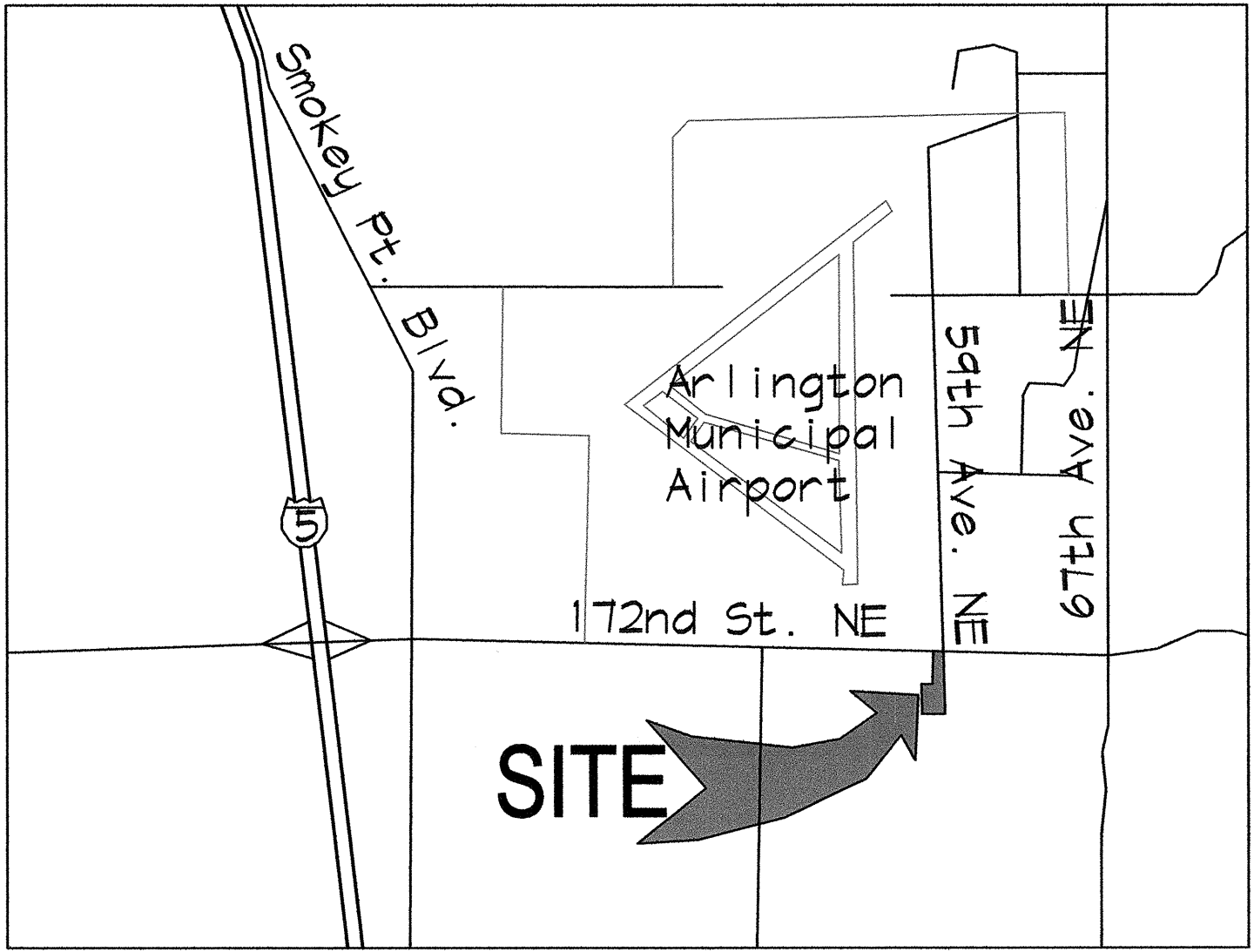
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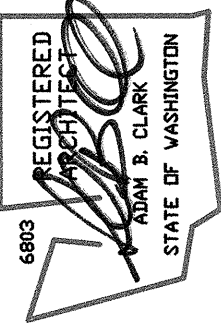
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VICINITY MAP



Date:	For:
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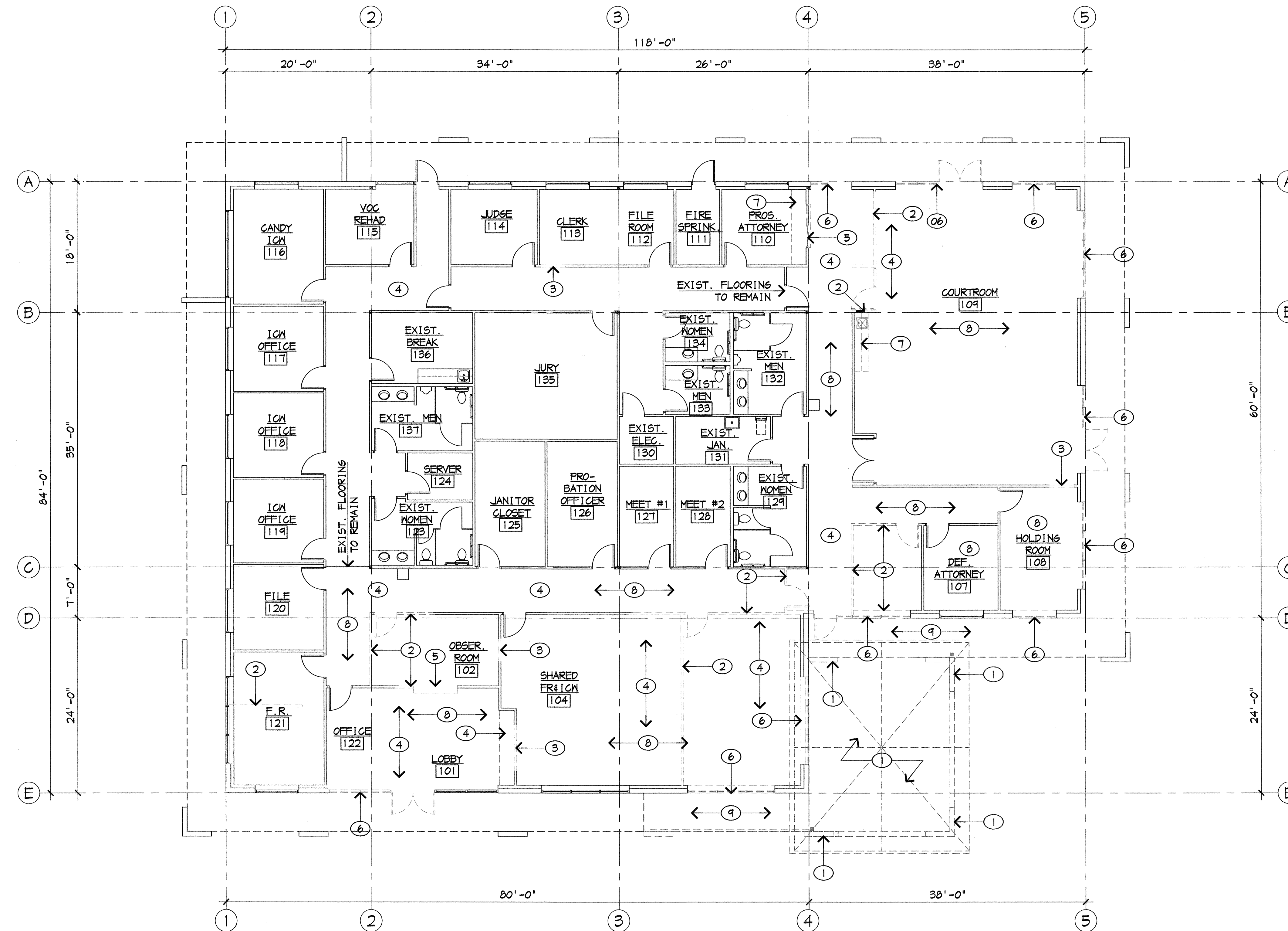
COVER DRAWING

Drawing:

A 0.1

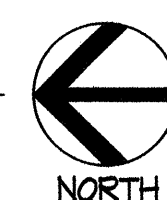
Job Number:

17c-3827



# DEMOLITION FLOOR PLAN

1/8" = 1'-0"

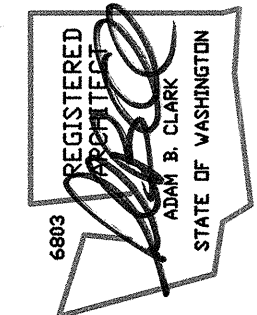


## DEMOLITION GENERAL NOTES

1. CONTRACTOR RESPONSIBLE FOR SITE INVESTIGATION PRIOR TO DEMOLITION TO REVEAL FULL SCOPE OF WORK. INVESTIGATE AND VERIFY LOCATIONS OF EXISTING STRUCTURAL, MECHANICAL, ELECTRICAL AND PLUMBING ELEMENTS AND NOTIFY THE ARCHITECT OF RECORD IN WRITING OF ANY AND ALL DISCREPANCIES PRIOR TO PROCEEDING WITH WORK.
2. IF DURING DEMOLITION CONDITIONS ARE REVEALED THAT MAY JEOPARDIZE INTEGRITY OF STRUCTURE OR PRECLUDE FOLLOWING DESIGN INTENT, CONTRACTOR SHALL NOTIFY THE ARCHITECT OF RECORD IMMEDIATELY.
3. CONTRACTOR TO LOCATE EXISTING WASTE WATER LINES, AND CLEAN OUT LINES PRIOR TO REUSE. VERIFY LINES ARE IN GOOD WORKING CONDITION. REPAIR LINES AS NECESSARY FOR NEW WORK.
4. PROTECT ALL EXISTING ITEMS AND CONSTRUCTION TO REMAIN AGAINST DAMAGE DURING DEMOLITION AND CONSTRUCTION.

## DEMOLITION PLAN NOTES

1. REMOVE EXISTING MASONRY AND STEEL TOWER COMPLETE.
2. REMOVE EXISTING INTERIOR WALL CONSTRUCTION AS REQUIRED TO ACCOMMODATE NEW CONSTRUCTION. THIS INCLUDES REMOVAL OF ALL DOOR, RELIGHTS ELECTRICAL AND OTHER INCORPORATED ITEMS WITHIN EXISTING WALL CONSTRUCTION.
3. REMOVE PORTION OF EXISTING WALL AS REQUIRED FOR INSTALLATION OF NEW DOOR OR RELIGHT. SEE FLOOR PLAN A2.1.
4. REMOVE PORTION OF EXISTING CEILING AS REQUIRED FOR NEW CONSTRUCTION. SEE DWG. A2.2.
5. REMOVE EXISTING RELIGHT AND SHELF.
6. REMOVE EXISTING STOREFRONT ASSEMBLY COMPLETE.
7. REMOVE EXISTING CASEWORK. CAP ALL PLUMBING & DRAIN LINES WITHIN WALL. PROVIDE ACCESS PANEL @ CAPPED LINE LOCATIONS.
8. REMOVE EXISTING FLOOR COVERINGS THROUGHOUT THIS SPACE.
9. REMOVE PORTION OF EXISTING CANOPY.



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DEMOLITION FLOOR PLAN

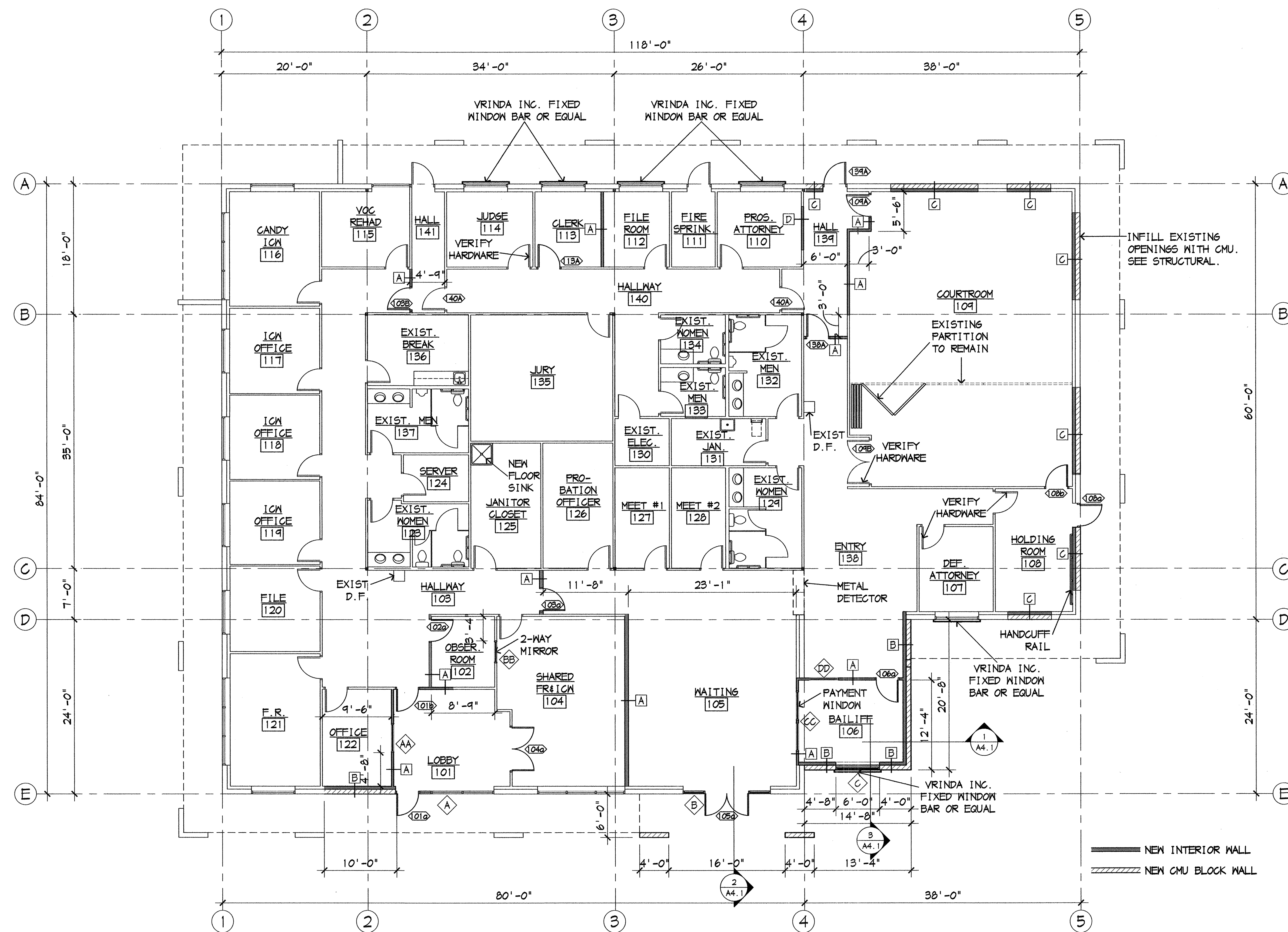
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**A 2.0**

Job Number:

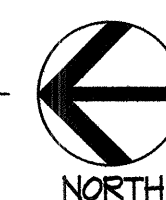
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# PROPOSED FLOOR PLAN

1/8" = 1'-0"



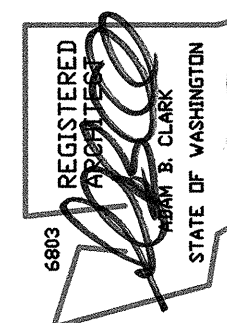
## WALL TYPES

- A** Construct wall from floor to underside of roof structure with 3625125-33 metal studs @ 16" o.c. with 5/8" GMB ea. side. Provide sound insulation in stud cavities.
- B** Furr wall from floor to underside of roof structure with 6005125-33 metal studs @ 24" o.c. with 5/8" GMB on interior side. Provide R-19 batt insulation in stud cavities & 4 mil vapor barrier @ warm side of furring below GMB.
- C** Furr infill portion of wall w/ 6005125-33 metal studs @ 24" o.c. w/ 5/8" GMB @ interior side. Provide R-19 batt insulation in stud cavities & 4 mil vapor barrier @ warm side of furring below GMB.
- D** Infill existing opening w/ studs & gmb to match existing.

## SYMBOLS LEGEND

- xxx** DOOR NUMBER. SEE DOOR SCHEDULE DWG. A5.1
- xxx** ROOM NUMBER. SEE FINISH SCHEDULE DWG. A5.1
- x** WALL TYPE. SEE THIS DRAWING.
- xx** RELIGHT. SEE WINDOW AND RELIGHT SCHEDULE DWG. A5.1

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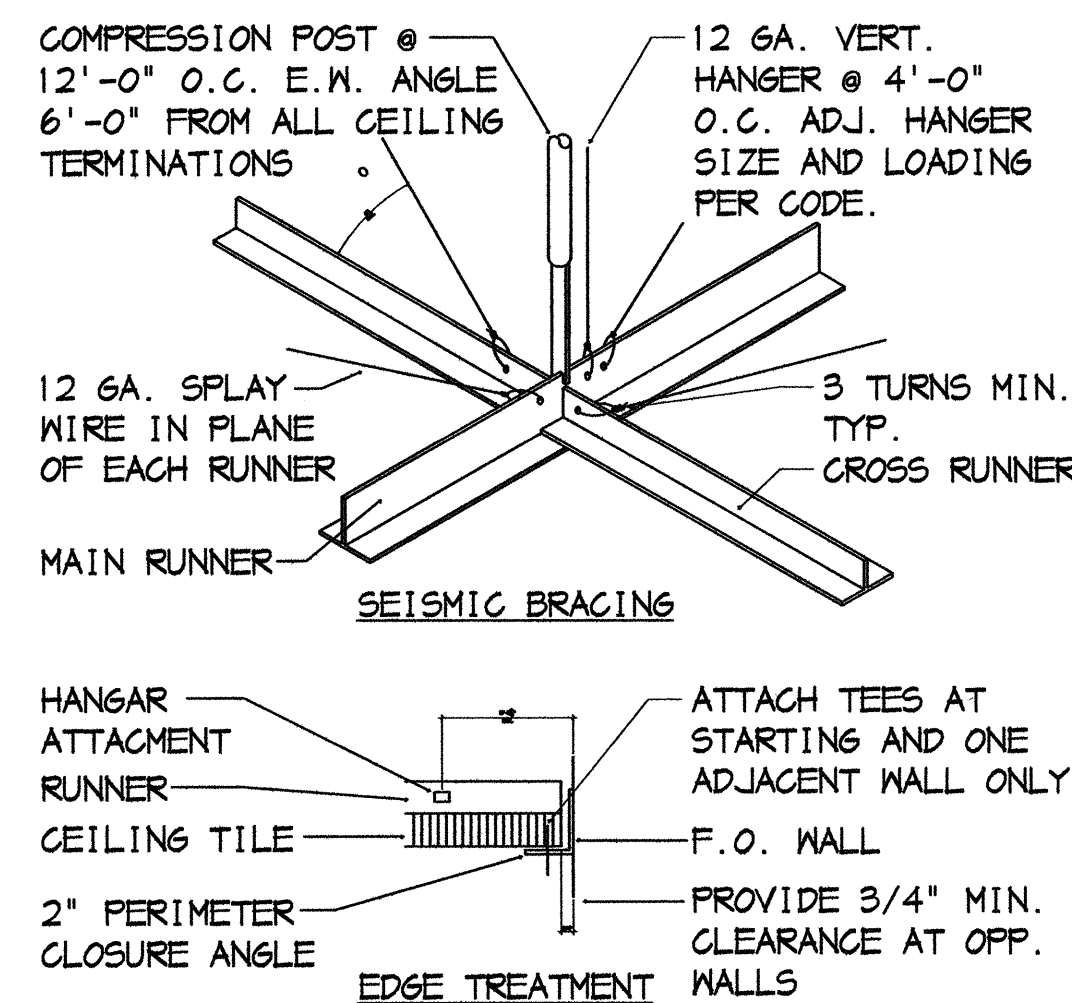
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PROPOSED FLOOR PLAN

Drawing:  
**A 2.1**  
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#### CODE REQUIREMENTS

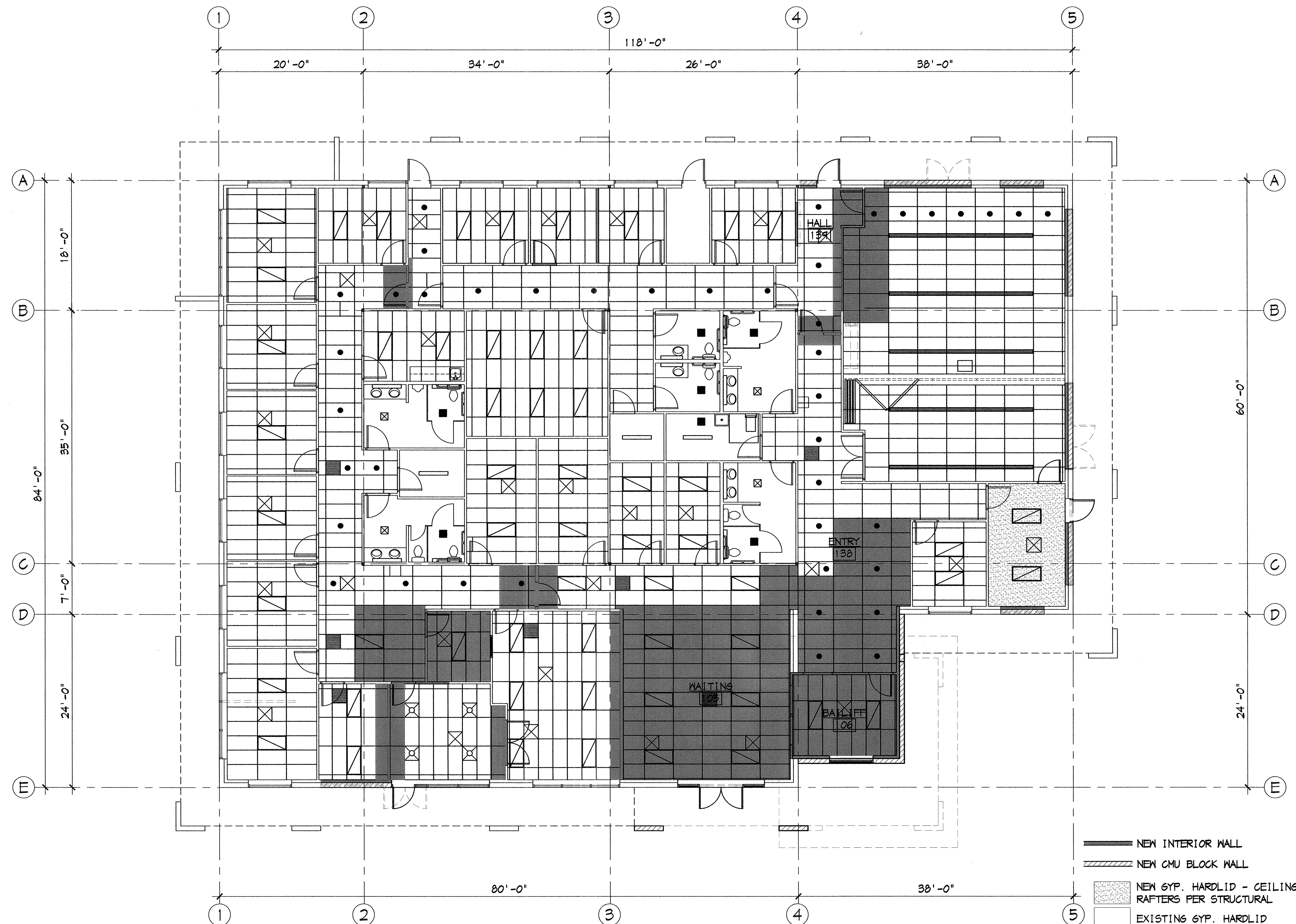
SUSPENDED CEILING SYSTEM TO COMPLY WITH:  
NORTHWEST WALL & CEILING BUREAU DOCUMENT 401.  
ASTM C 635, ASTM C 636, ASTM E580, SECTION 5 - SEISMIC  
DESIGN CATEGORIES D, E, AND F OF ASCE 7-10.

ACOUSTICAL TILE OR LAY-IN PANEL CEILINGS SHALL ALSO COMPLY WITH THE FOLLOWING:

A. THE WIDTH OF THE PERIMETER SUPPORTING CLOSURE SHALL BE NOT LESS THAN 2". IN EACH ORTHOGONAL HORIZONTAL DIRECTION, ONE END OF THE CEILING GRID SHALL BE ATTACHED TO THE CLOSURE ANGLE. THE OTHER END IN EACH HORIZONTAL DIRECTION SHALL HAVE A .75" CLEARANCE FROM THE WALL AND SHALL REST UPON AND BE FREE TO SLIDE UPON A CLOSURE ANGLE OR CHANNEL.

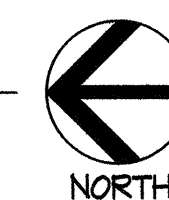
B. FOR CEILING AREAS EXCEEDING 2,500 SQUARE FEET, A SEISMIC SEPARATION JOINT OR FULL HEIGHT PARTITION THAT BREAKS THE CEILING INTO AREAS LESS THAN 2,500 SQUARE FEET EACH WITH A RATIO OF THE LONG SHORT DIMENSION LESS THAN OR EQUAL TO 4, SHALL BE PROVIDED UNLESS STRUCTURAL ANALYSES ARE PERFORMED OF THE CEILING BRACING SYSTEM FOR THE PRESCRIBED SEISMIC FORCES THAT DEMONSTRATE THAT CEILING PENETRATIONS AND CLOSURE ANGLES OR CHANNELS PROVIDE SUFFICIENT CLEARANCE TO ACCOMMODATE THE ANTICIPATED LATERAL DISPLACEMENT. EACH AREA SHALL BE PROVIDED WITH CLOSURE ANGLES OR CHANNELS IN ACCORDANCE WITH SECTION 13.5.6.2.2.A AND HORIZONTAL RESTRAINTS OR BRACING. SECTION 13.5.6.2.2.

1 CEILING BRACING DTL  
NO SCALE

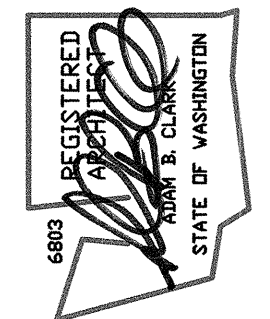


#### REFLECTED CEILING PLAN

1/8" = 1'-0"



- NEW INTERIOR WALL
- NEW CMU BLOCK WALL
- NEW GYP. HARDLID - CEILING RAFTERS PER STRUCTURAL
- EXISTING GYP. HARDLID
- NEW OR REPLACED A.C.T.
- EXISTING A.C.T.
- NEW OR EXISTING AIR EXCHANGE VENT
- NEW OR EXISTING RETURN AIR



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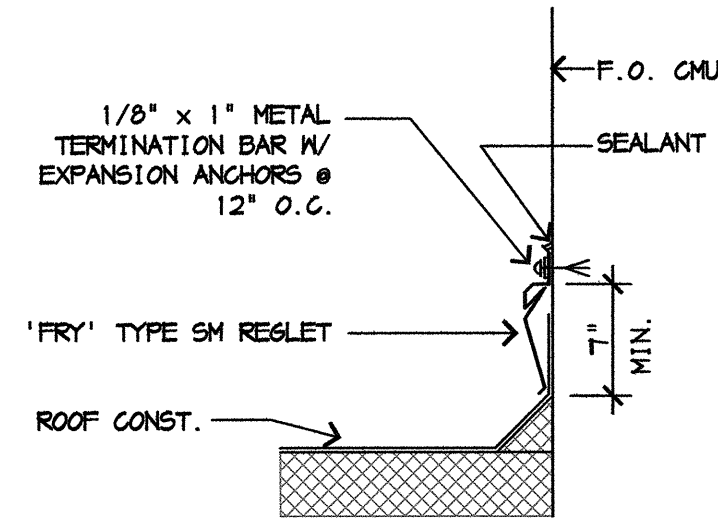
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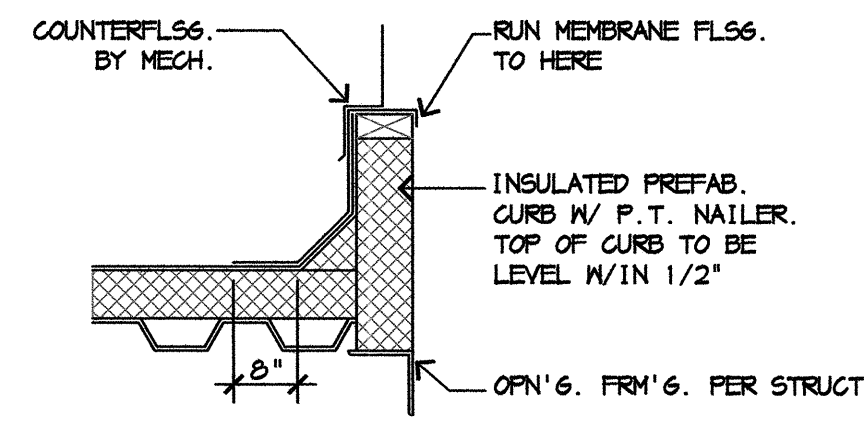
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REFLECTED CEILING PLAN

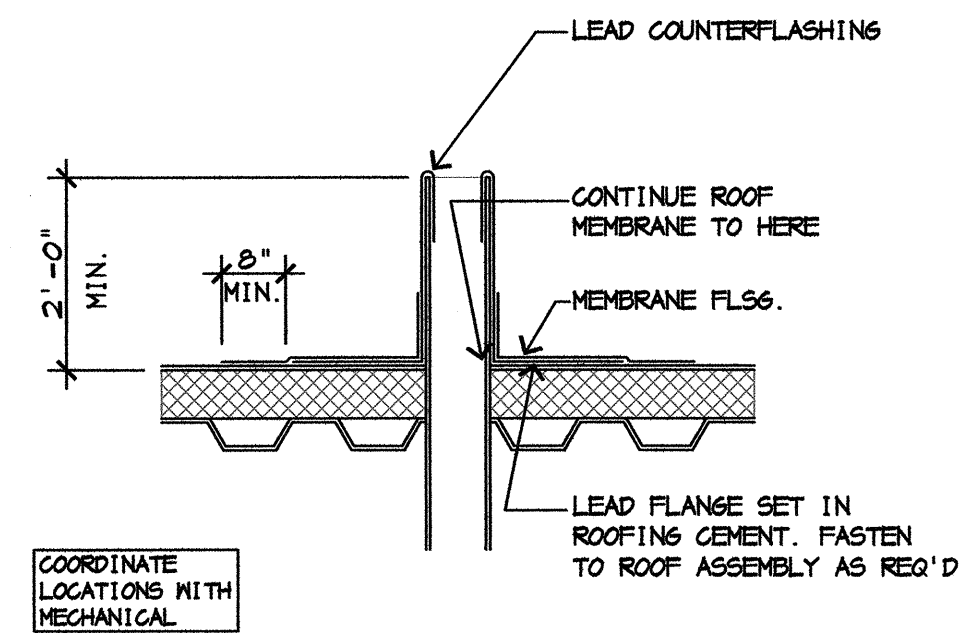




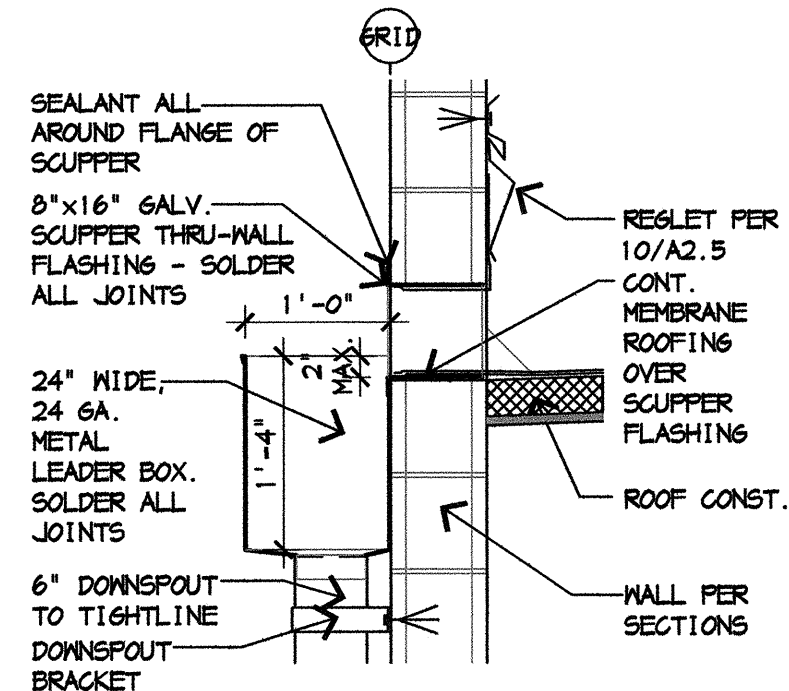
1 REGLET  
1" = 1'-0"



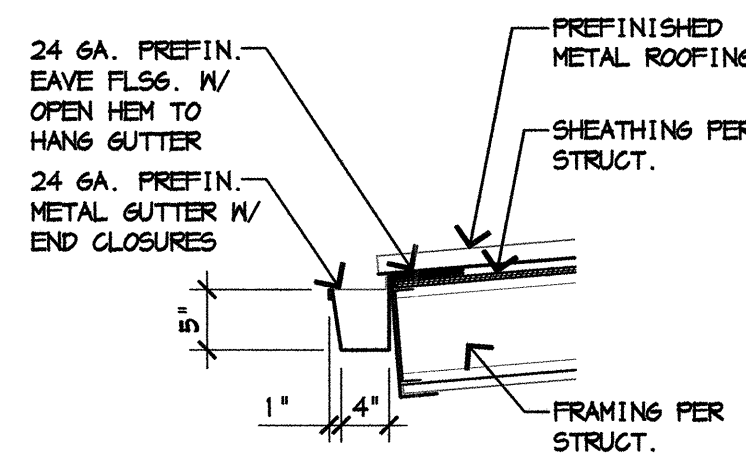
2 ROOF CURB  
1/2" = 1'-0"



3 PLUMBING VENT  
1/2" = 1'-0"

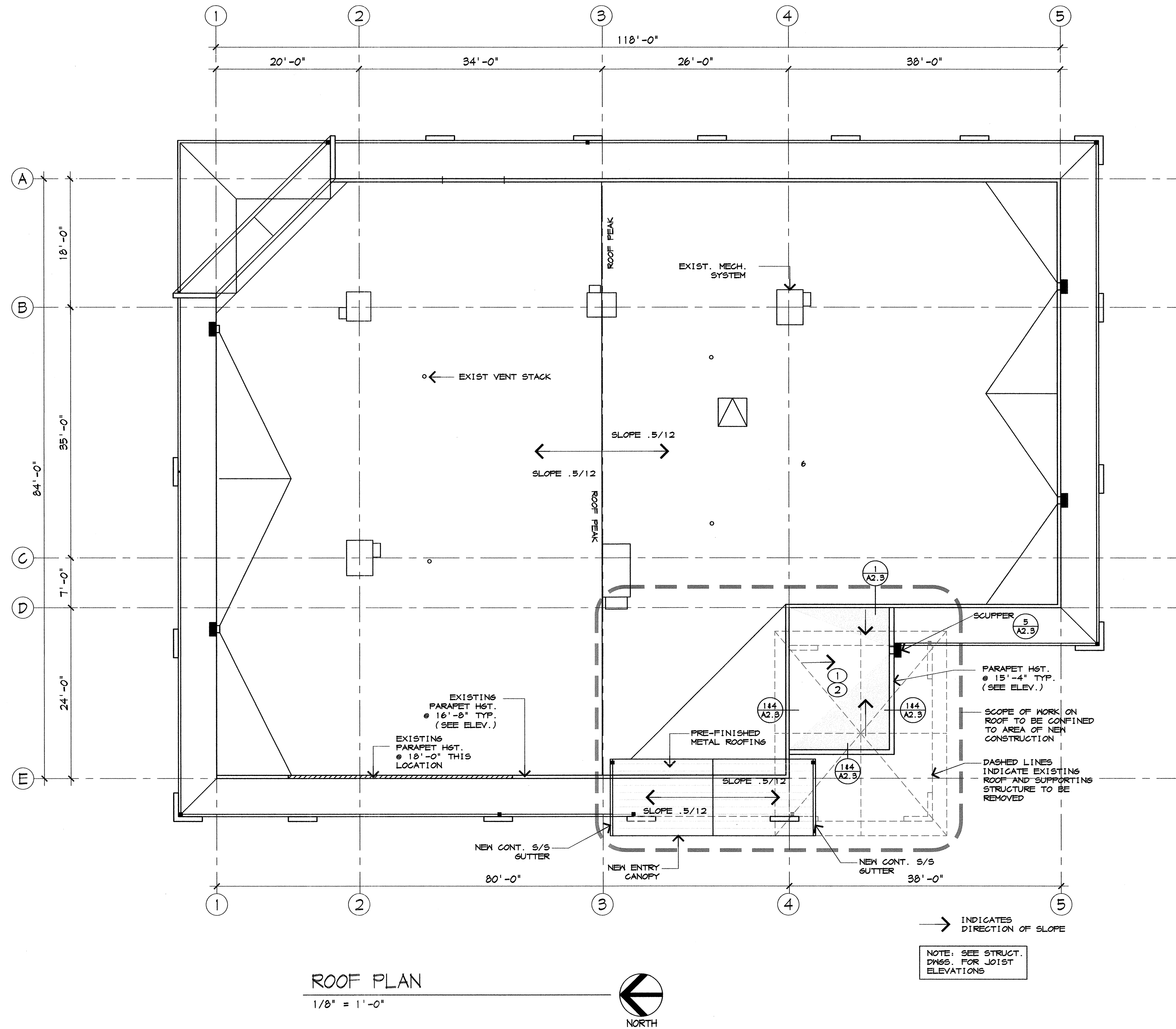


4 COPING  
1" = 1'-0"

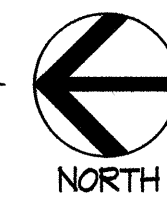


5 LEADER BOX  
1" = 1'-0"

6 CANOPY GUTTER  
1" = 1'-0"



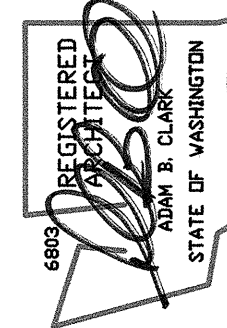
ROOF PLAN  
1/8" = 1'-0"



#### ROOF PLAN NOTES

1. ROOF ELEVATIONS ARE MEASURED TO UNDERSIDE OF ROOF DECK (TOP OF JOISTS). SEE STRUCT. DWGS.
2. ROOF CONSTRUCTION: MEMBRANE ROOFING ON R-50 RIGID INSULATION OVER METAL DECKING.
3. PLUMBING AND GAS VENT PENETRATIONS THROUGH ROOF. SEE DTL. 3 DWG. A2.3  
-COORDINATE LOCATIONS WITH MECHANICAL.
4. SHADING DENOTES AREAS OF ADDITIONAL TAPERED RIGID INSULATION ABOVE REQUIRED INSULATION. PROVIDE ADEQUATE INSULATION AS REQUIRED TO ACHIEVE SLOPE OF 1/4" PER FOOT TO ROOF DRAIN. HVAC UNIT CURBS. VERIFY SIZE AND LOCATION WITH MECH.
5. TAPERED INSULATION CRICKET. INSTALL AS REQUIRED TO DIRECT WATER FLOW AROUND.

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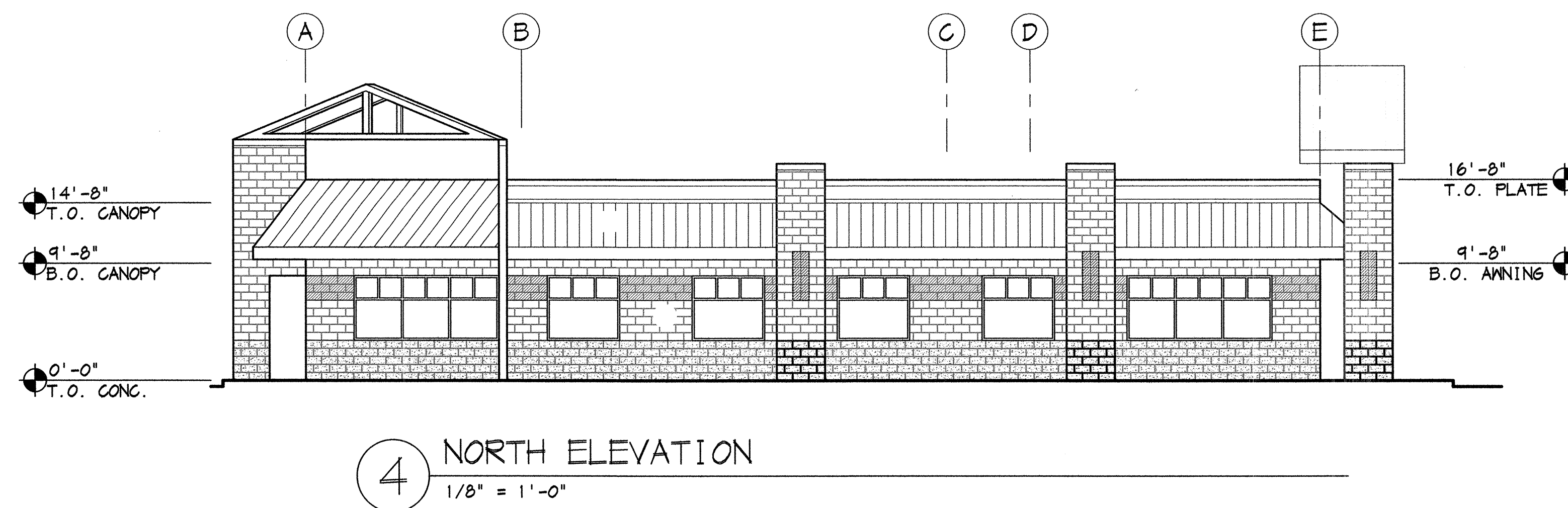
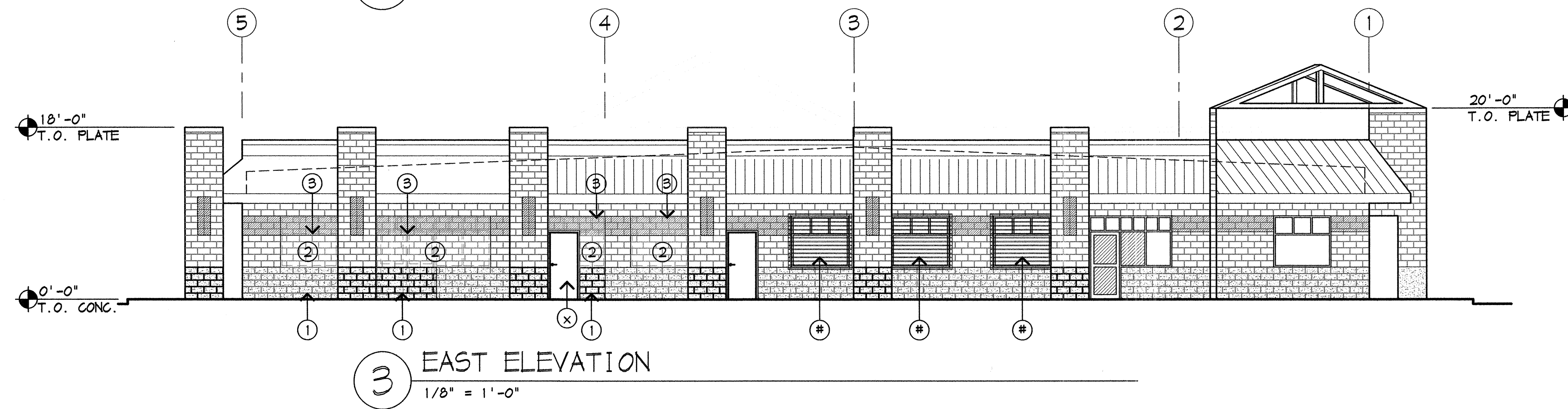
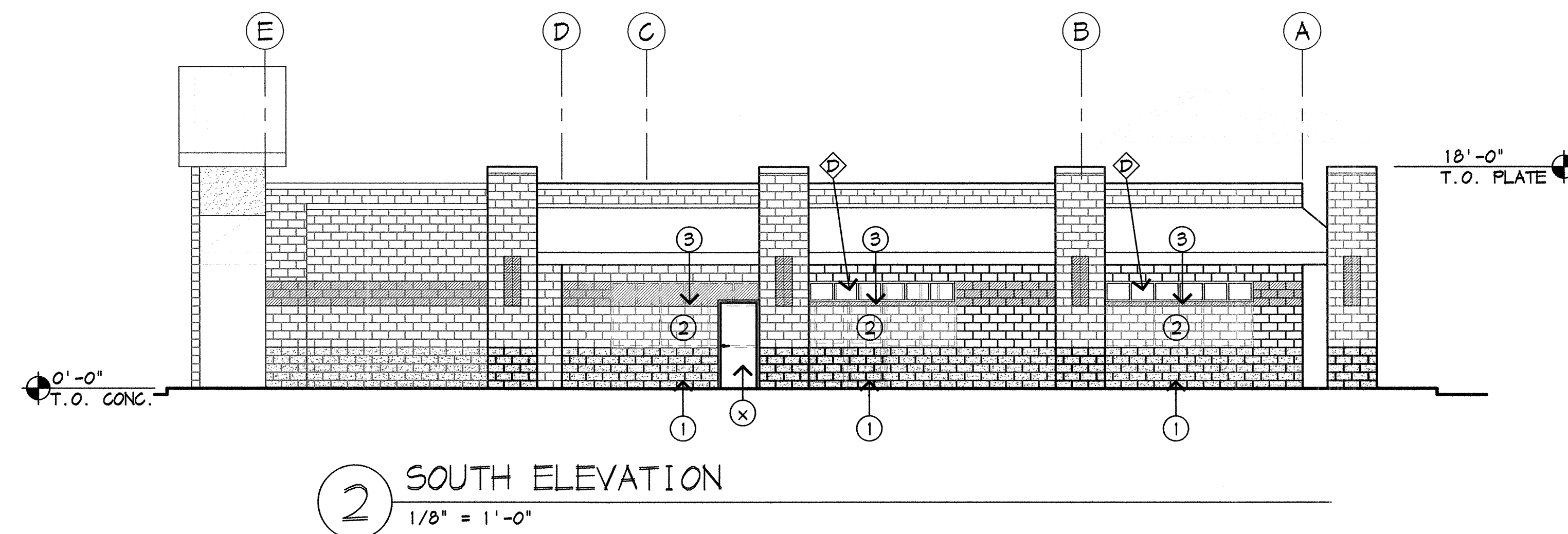
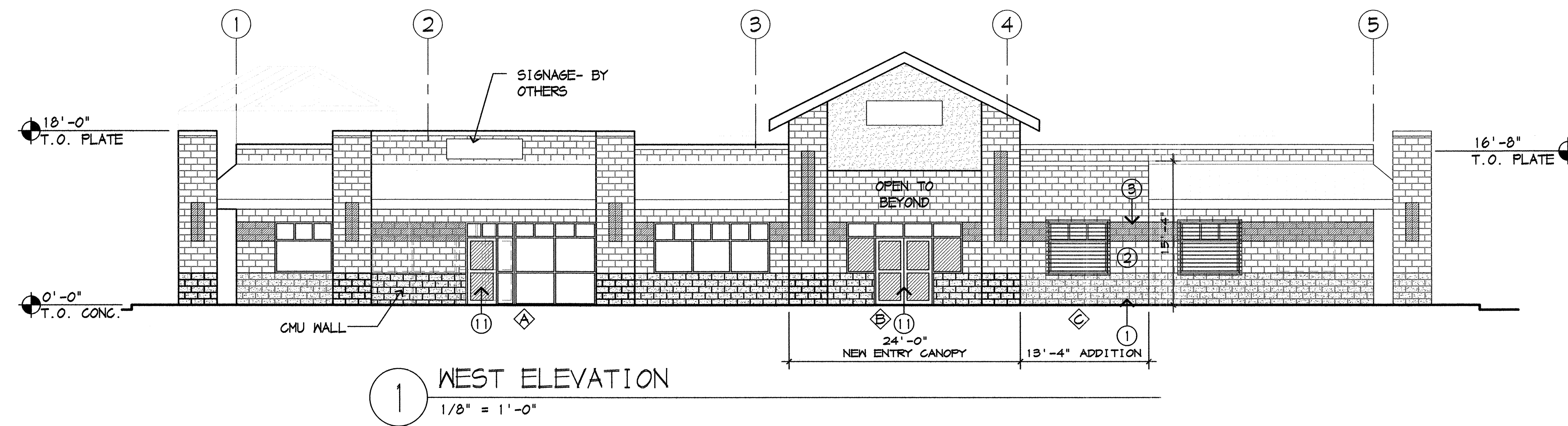
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Comments:  
ROOF PLAN



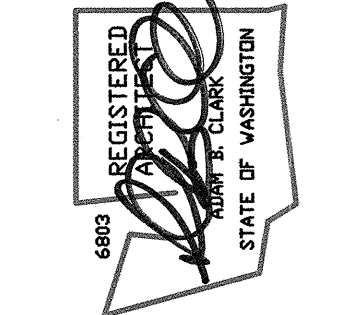


## ELEVATION NOTES

- 8 X 8 X 16 SPLIT FACE CMU, COLOR C-2.
- 8 X 8 X 16 SMOOTH FACE CMU, COLOR C-1.
- 8 X 8 X 16 SMOOTH FACE CMU, COLOR C-3.
- 8 X 8 X 16 1 SCORE SMOOTH FACE CMU, COLOR C-3.
- ROOF LINE.
- INSULATED METAL DOOR AND FRAME, MATCH COLOR C-4.
- METAL CANOPY PER STRUCTURAL, PAINT COLOR C-4.
- CONT. STAINLESS STEEL GUTTER.
- STAINLESS STEEL SCUPPER, SEE DET.
- 1" INSULATED GLASS IN NATURAL ANODIZED ALUMINUM STOREFRONT FRAMING. SEE WALL SECTIONS. PROVIDE 1" INSULATED SAFETY GLASS WHERE SHOWN.
- STOREFRONT ENTRY DOOR, COLOR TO MATCH STOREFRONT FRAMING. SEE DOOR SCHEDULE.
- PREFINISHED METAL ROOFING, COLOR C-4.
- PREFINISHED METAL COPING, COLOR C-4.
- STUCCO FINISH OVER GYPSUM SHEATHING- COLOR C-5.
- PREFINISHED METAL EAVE, COLOR C-4.
- PREFINISHED METAL SOFFIT, COLOR C-4.
- METAL TRUSS- PAINT C-4.

## ELEVATION COLORS

- C-1 "MUTUAL MATERIALS" COCOA  
C-2 "MUTUAL MATERIALS" MOUNTAIN BROWN  
C-3 "MUTUAL MATERIALS" NATURAL  
C-4 "AEP SPAN" COOL METALLIC SILVER



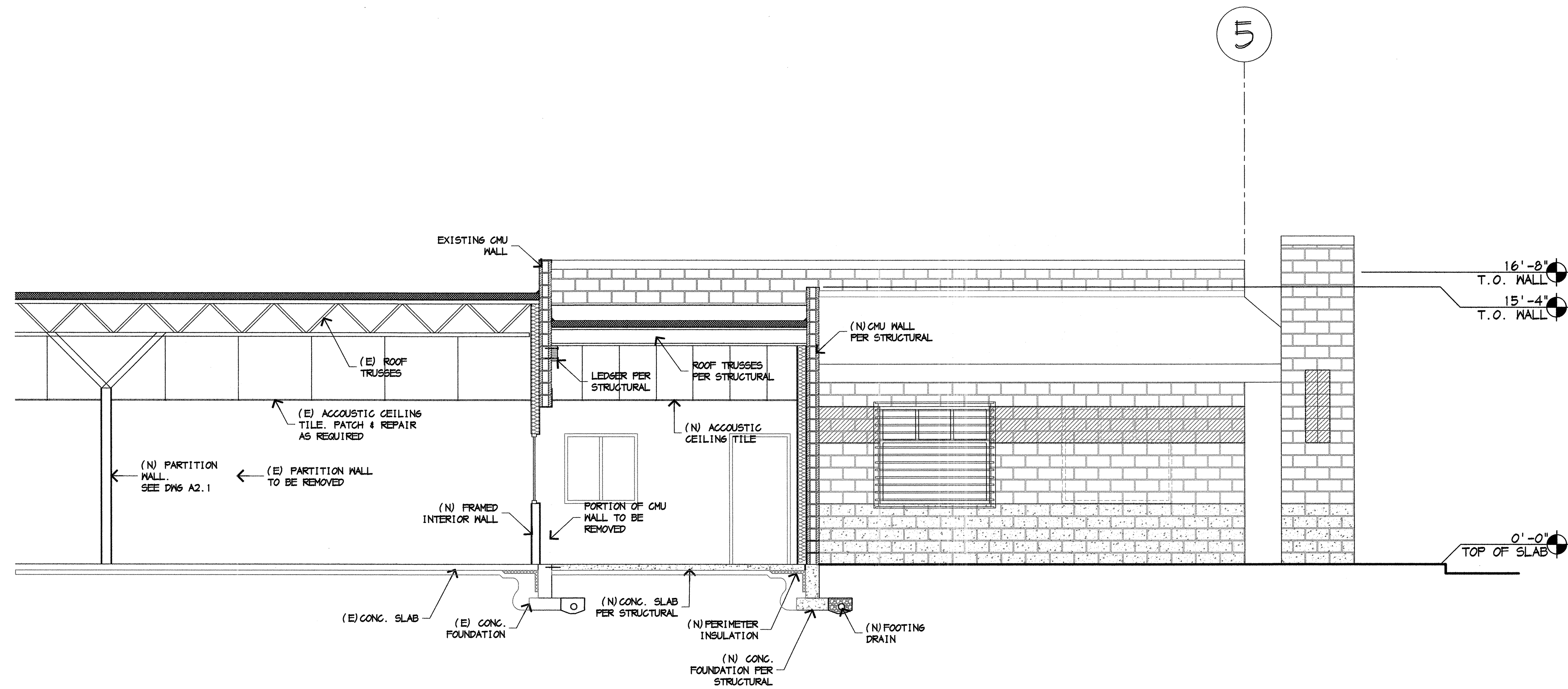
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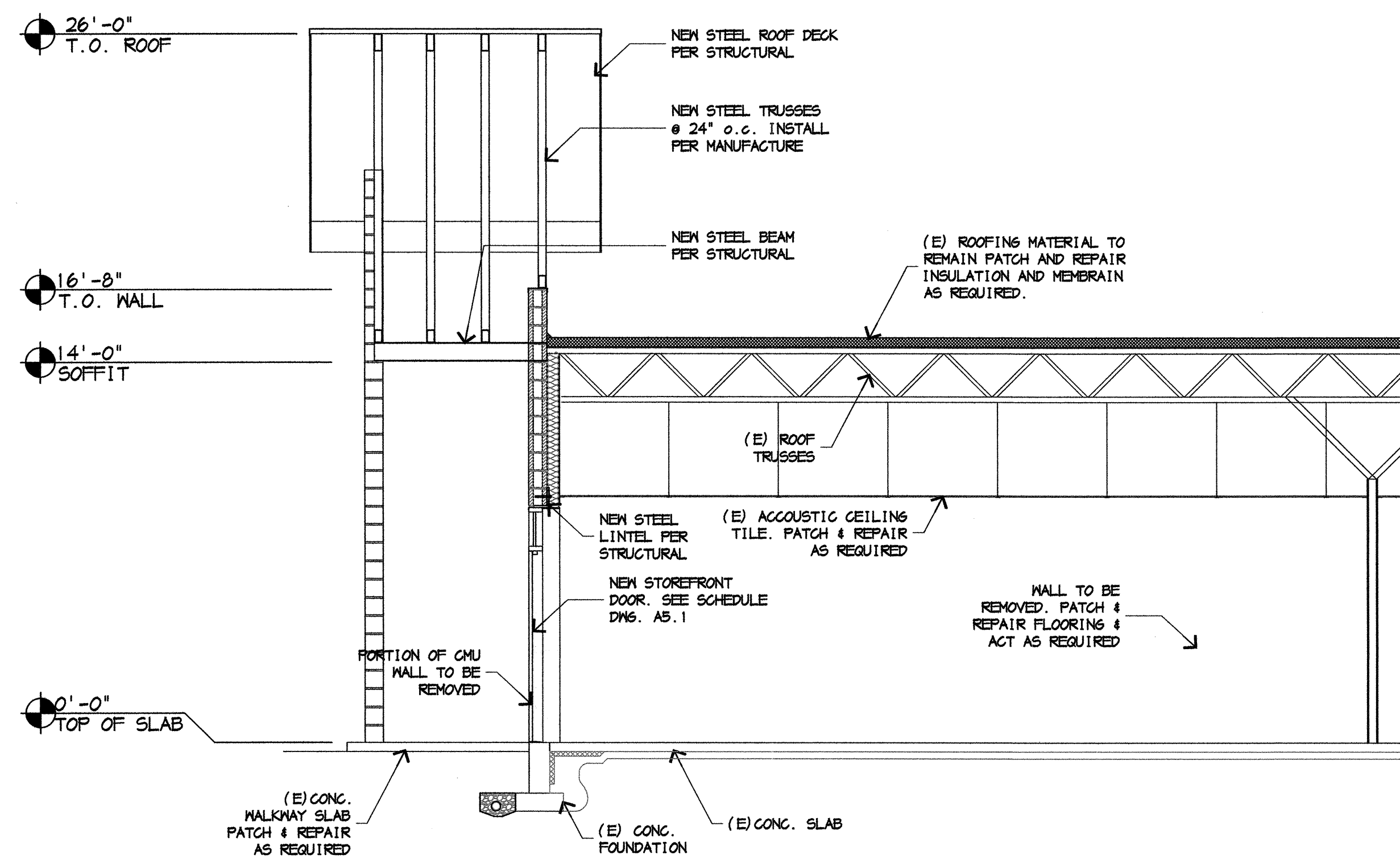
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Comments:  
EXTERIOR ELEVATIONS

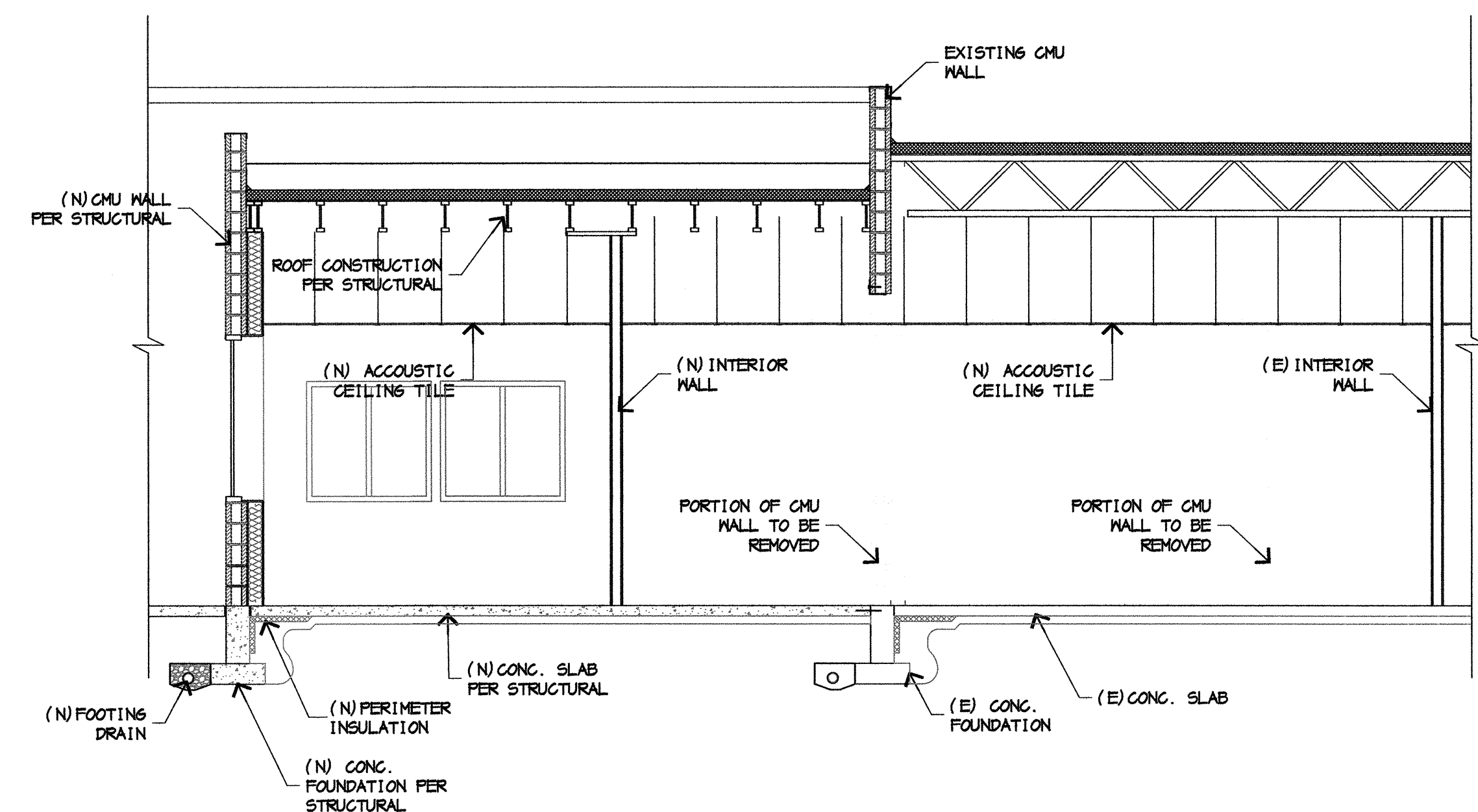




1 Building Section Scale: 1/4" = 1'-0"

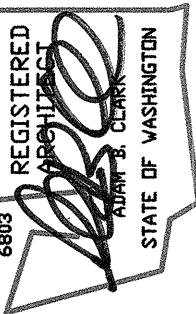


2 Building Section Scale: 1/4" = 1'-0"



3 Building Section Scale: 1/4" = 1'-0"

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BUILDING SECTIONS

Drawing:  
**A4.1**  
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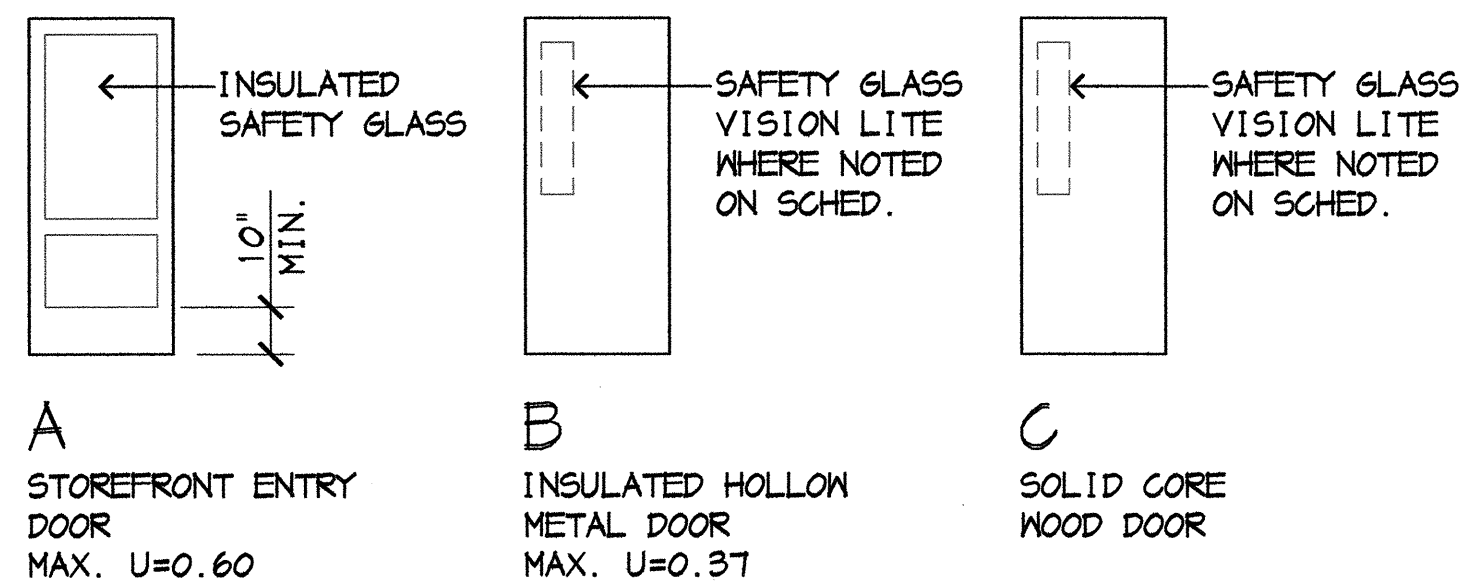


DOOR SCHEDULE						
DOOR #	DOOR SIZE (W X H)	DOOR TYPE	JAMB TYPE	H.W. GROUP	FIRE RATING	NOTES ① ② ③
101A	3'-0" X 7'-0"	A	I	1	-	NOTE: 4,5
101B	3'-0" X 7'-0"	C	III	4	-	-
102A	3'-0" X 7'-0"	C	III	5	-	-
103A	3'-0" X 7'-0"	C	III	4	-	-
103B	3'-0" X 7'-0"	C	III	4	-	-
104A	3'-0" X 7'-0" PAIR	C	III	6	-	NOTES: 6
105A	3'-0" X 7'-0" PAIR	A	I	2	-	NOTES: 4,5
106A	3'-0" X 7'-0"	C	III	5	-	NOTES: 6
108A	3'-0" X 7'-0"	B	II	3	-	-
108B	3'-0" X 7'-0"	C	III	4	-	-
109A	3'-0" X 7'-0"	C	III	4	-	-
109B	3'-0" X 7'-0"	C	III	4	-	-
113A	3'-0" X 7'-0"	C	III	5	-	-
138A	3'-0" X 7'-0"	C	III	5	-	-
140A	3'-0" X 7'-0"	C	III	5	-	-
140B	3'-0" X 7'-0"	C	III	5	-	-

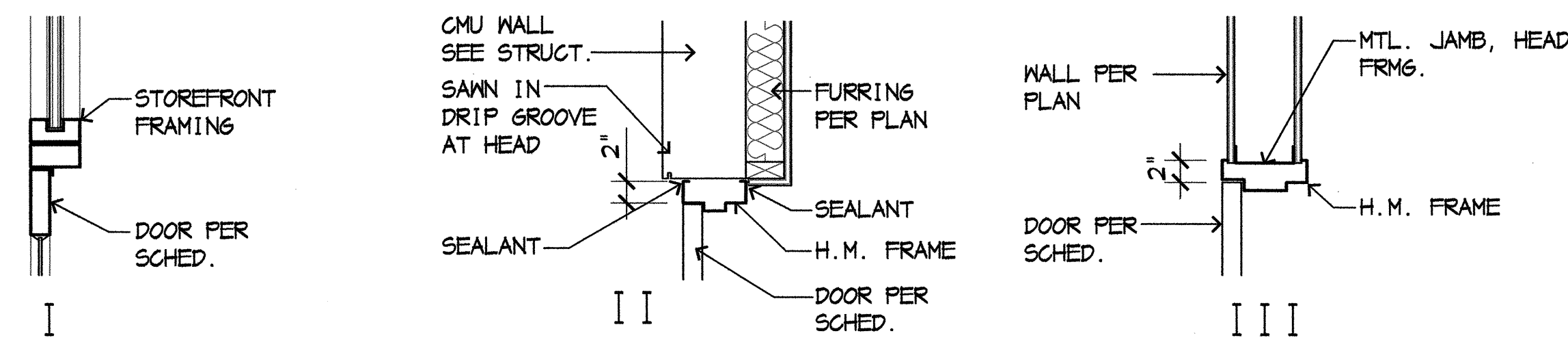
### DOOR NOTES

- ALL HARDWARE FINISHES TO BE US26D.
- FURNISH DOOR STOPS AS REQUIRED AT ALL DOORS.
- PROVIDE LEVER HANDLES ON ALL LOCKSETS AND LATCHSETS.
- ALL PANIC HARDWARE TO BE INSTALLED 30"-44" A.F.F.
- ADA EXIT SIGNAGE WITH TACTILE LETTERS AND BRAILLE MOUNTED ON LATCH SIDE OF SINGLE DOOR OR RIGHT OF R.H. DOOR AT ACTIVE DOUBLE DOORS. WHERE THERE IS NO WALL SPACE ON LATCH SIDE OF SINGLE DOOR OR RIGHT SIDE OF DOUBLE DOORS, SIGN SHALL BE ON THE NEAREST ADJACENT WALL. EXCEPTION: DOOR MOUNTED SIGN PERMITTED ON PUSH SIDE OF DOOR W/ CLOSER WITHOUT HOLD OPEN DEVICE. RE: FIXTURE HEIGHT SCHEDULE ON DWG. A5.1.
- VISION LITE SIZE TO BE DETERMINED BY OWNER.

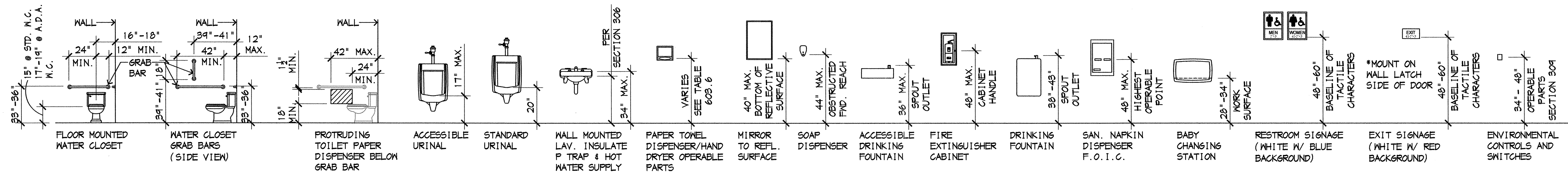
### DOOR TYPES



### JAMB TYPES



NOTE: FOR TABLE AND SECTION REFERENCES SEE ICC A117.1-2009

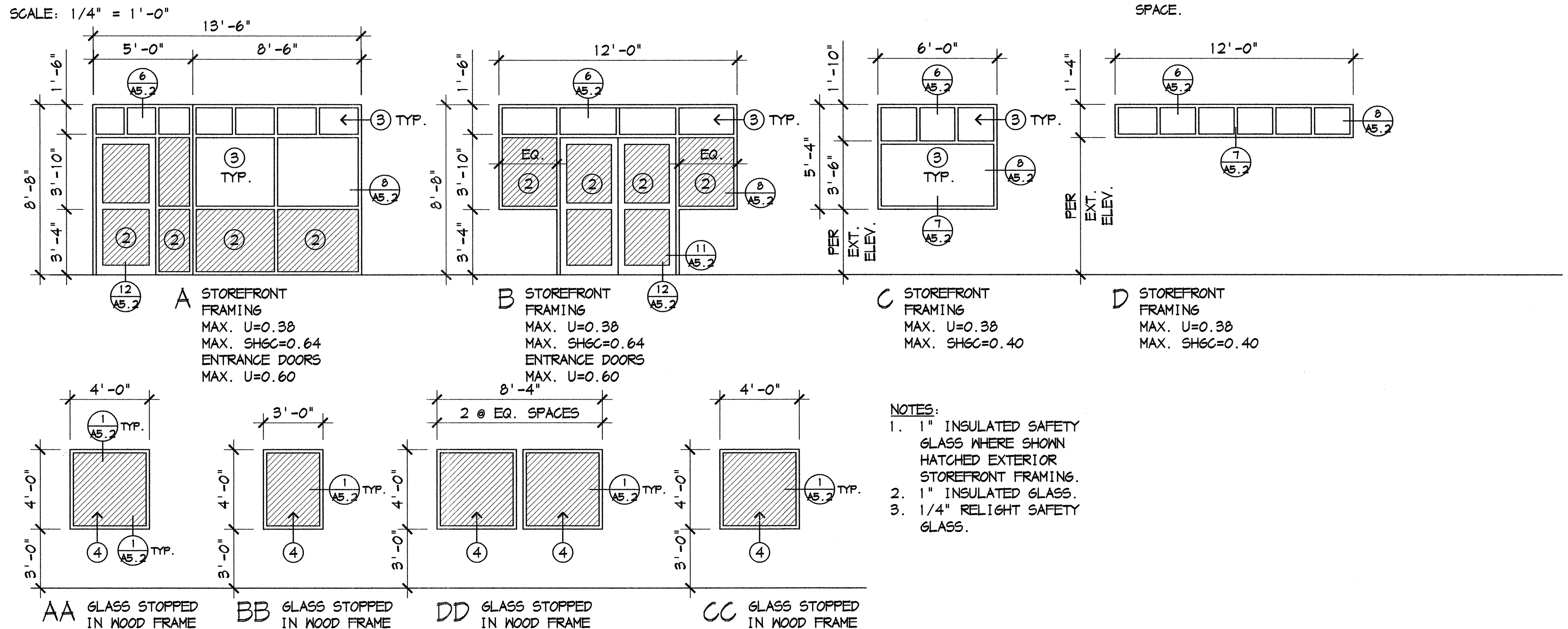


### FIXTURE HEIGHT DIAGRAM

### HARDWARE GROUPS

- PIVOTS AS REQUIRED  
1 LOCKSET  
1 PUSH BAR PANIC DEVICES  
1 CLOSERS (SIZE TO SUIT DOORS)  
1 FULL HANDLES  
WEATHERSTRIPPING & SNEEP  
THRESHOLD (FULL WIDTH OF OPENING)
- PIVOTS AS REQUIRED  
1 LOCKSET  
2 PUSH BAR PANIC DEVICE  
2 CLOSERS (SIZE TO SUIT DOORS)  
WEATHERSTRIPPING & SNEEP  
THRESHOLD (FULL WIDTH OF OPENING)
- 1 1/2 PAIR BUTTS  
1 LOCKSET  
1 CLOSER (SIZE TO SUIT DOOR)  
1 KICKPLATE (PUSH SIDE OF DOOR)  
WEATHERSTRIPPING & SNEEP  
THRESHOLD (FULL WIDTH OF OPENING)
- 1 1/2 PAIR BUTTS  
1 LOCKSET  
1 CLOSER (SIZE TO SUIT DOOR)
- 1 1/2 PAIR BUTTS  
1 LOCKSET
- 3 PAIR BUTTS  
1 LOCKSET  
FLUSHBOLTS (INACTIVE LEAF)  
ASTRASAL

### WINDOW AND RELIGHT SCHEDULE



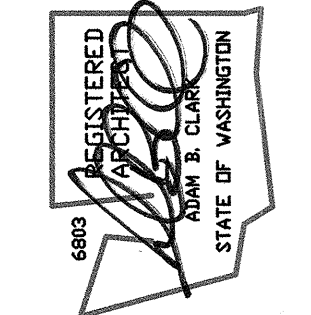
ROOM #	ROOM NAME	FLOOR	BASE	WALLS				MANSICOT	CEILING	CEILING HEIGHT	NOTES ① TYP
				NORTH	EAST	SOUTH	WEST				
101	LOBBY	F-2	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
102	OBSERVATION ROOM	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
103	HALLWAY	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
104	SHARED FR & ICM	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
105	WAITING	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
106	BALIFF	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1	8'-0"	NOTE: 2,3
107	DEF. ATTORNEY	F-1	B-1	M-1	M-1	M-1	M-1	--	C-2	--	NOTE: 2,3
108	HOLDING ROOM	F-1	B-1	M-1	M-1	M-1	M-1	--	C-2	--	NOTE: 2,3
109	COURT ROOM	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
110	PROS. ATTORNEY	F-1	B-1	M-1	M-1	M-1	M-1	--	C-2	--	NOTE: 2,3
112	FILE ROOM	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
113	CLERK	F-1	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
121	F.R.	F-1	B-1	M-1	M-1	M-1	M-1	--	C-2	--	--
122	OFFICE	F-1	B-1	M-1	M-1	M-1	M-1	--	C-2	--	--
125	JANITOR CLOSET	F-4	B-1	M-2	M-2	M-2	M-2	WC-1	C-5	--	NOTE: 6
138	ENTRY	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
139	HALLWAY	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
140	HALLWAY	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3
141	HALLWAY	F-3	B-1	M-1	M-1	M-1	M-1	--	C-1,2	--	NOTE: 2,3

### FINISH MATERIALS

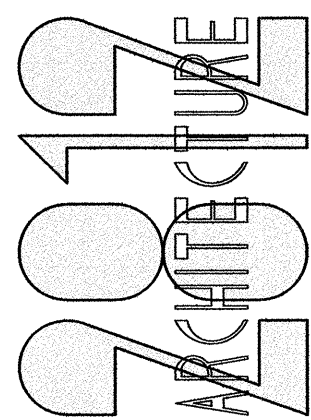
- FLOOR MATERIALS**  
F-1 CARPET TILE  
F-2 VINYL COMPOSITE TILE  
F-3 QUARRY TILE  
F-4 SEALED CONCRETE  
F-5 SEALED PLYWOOD
- WALL FINISHES**  
M-1 EGGSHELL ENAMEL  
M-2 SEMI-GLOSS ENAMEL
- CEILING**  
C-1 ACOUSTIC CEILING TILE  
C-2 EXISTING TO REMAIN  
C-3 SEMI-GLOSS ENAMEL  
C-4 EXPOSED FRAMING  
C-5 PATCH & REPAIR TO MATCH EXISTING
- MANSICOT**  
MC-1 4'-0" PLASTIC LAMINATE ALUM. EDGE TRIM @ ALL EXPOSED EDGES

### FINISH NOTES

- ALL WOOD DOORS AND FRAMES TO BE STAINED. ALL METAL DOORS AND FRAMES TO BE PAINTED. REUSE EXISTING CEILING TILE & GRID AS FEASIBLE. SEE REFLECTED CEILING PLAN DWG. A2.3
- SEE DET. 1 DWG. A2.2 FOR SUSPENDED ACOUSTICAL CEILING SEISMIC BRACING.
- CONSTRUCT CLG. PER STRUCT. W/ 5/8" GMB ATTACHED TO UNDERSIDE OF JOISTS OR STRUCTURE ABOVE.
- CONSTRUCT CLG. W/ 600S125-33 METAL JOISTS @ 16" O.C. W/ 5/8" GMB ATTACHED TO UNDERSIDE OF JOISTS.
- USE WATER RESISTANT GMB AT ALL WALLS WITHIN THIS SPACE.
- SEE FLOOR PLAN FOR LOCATION OF INTERIOR ELEVATIONS.
- PAINT ALL GMB WALLS WITHIN THIS SPACE.
- INSTALL BASE AT ALL WALLS WITH GMB WITHIN THIS SPACE.



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**Stillaguamish Tribal Courthouse**

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Arlington, Washington

Drawing:

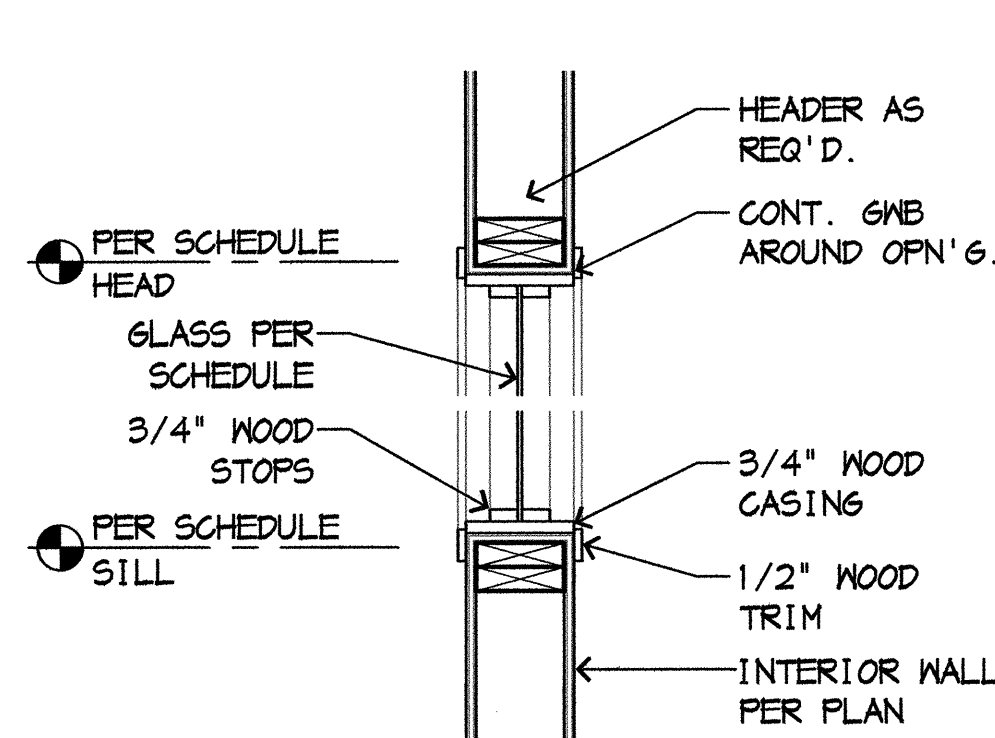
**A5.1**

Job Number:

17c-3827

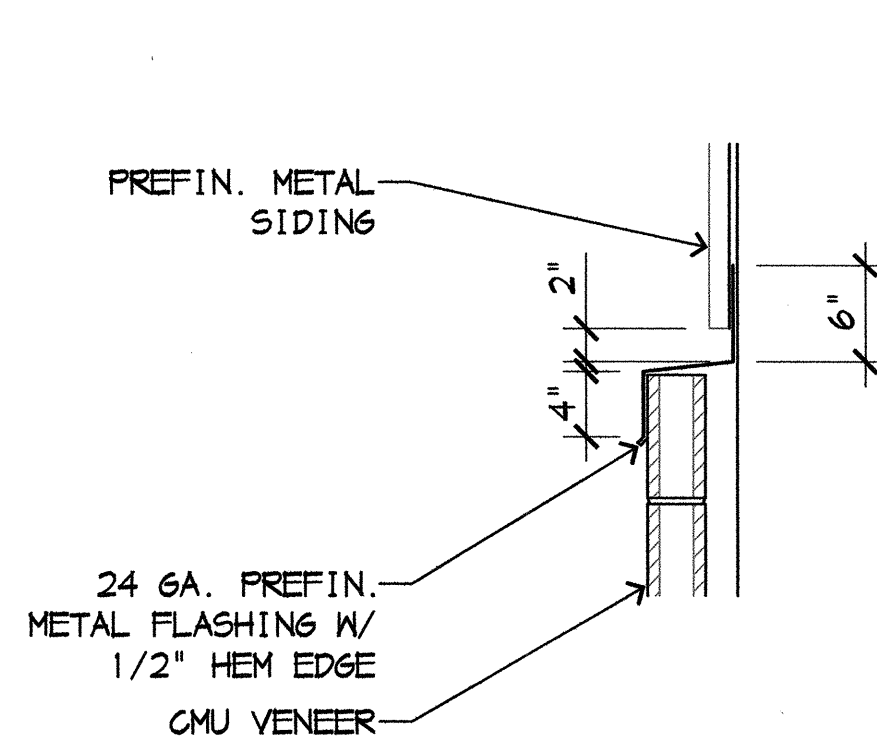
DOOR & FINISH SCHEDULES



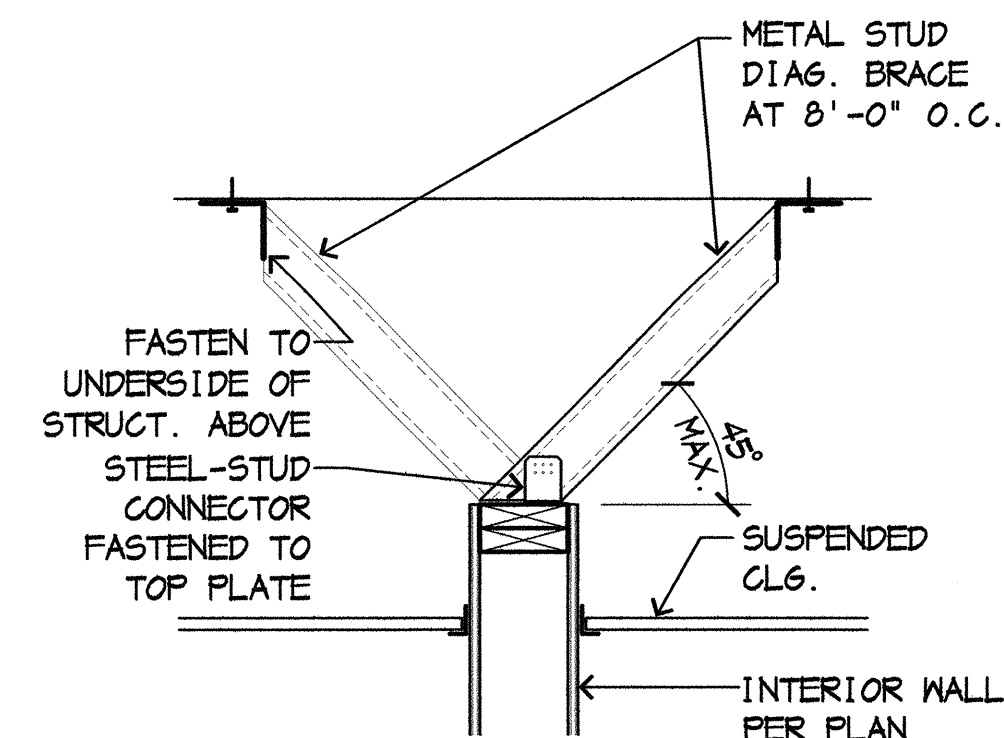


NOTE: GMB AND TRIM CONDITION AT SILL, JAMB, AND HEAD IDENTICAL

1 RELIGHT  
1" = 1'-0"

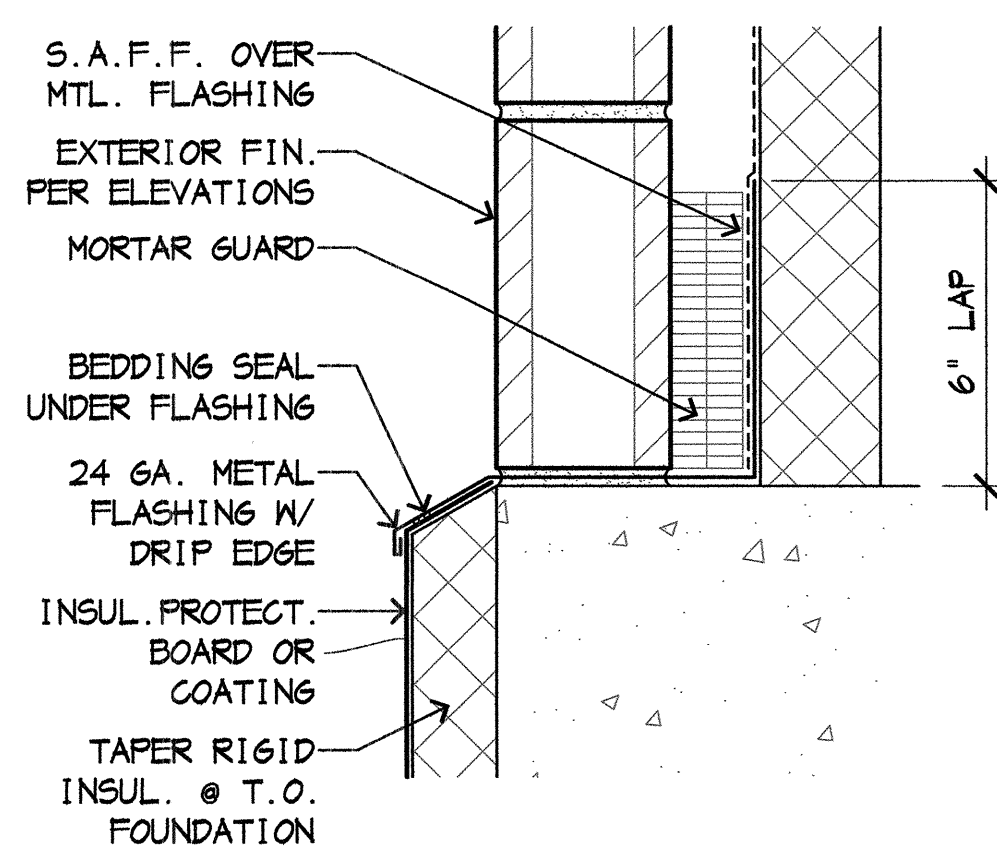


2 MTL. PANEL @ CMU  
1" = 1'-0"

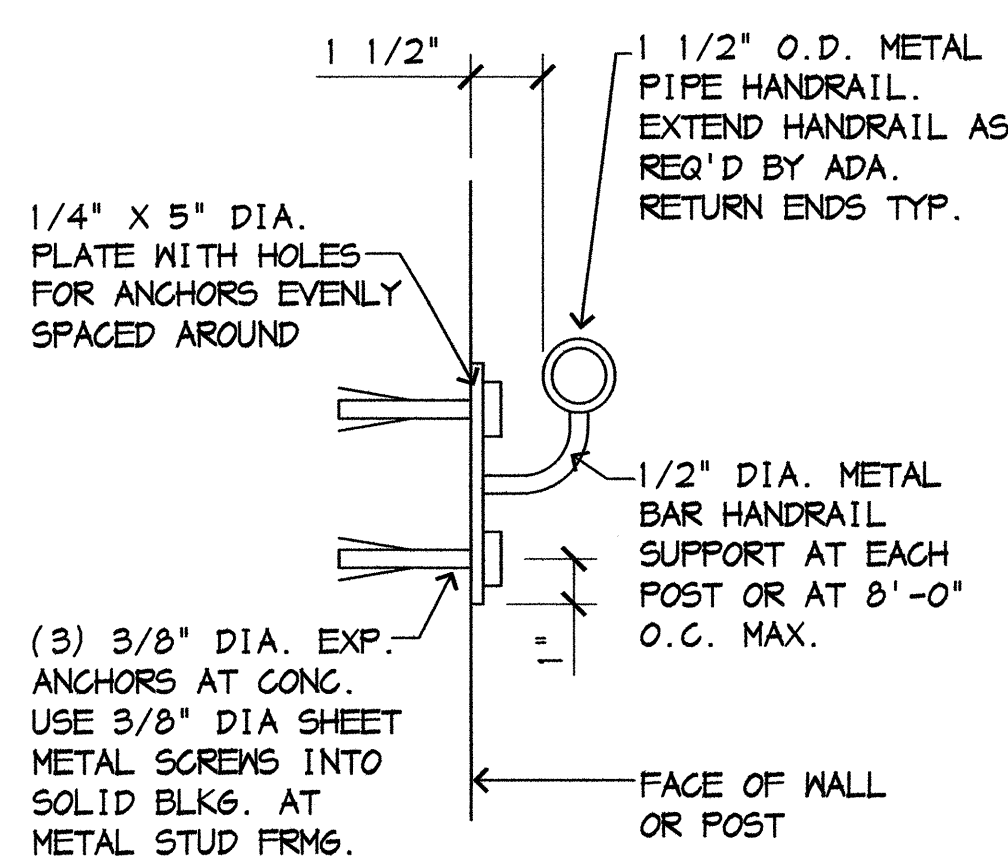


NOTE: INSTALL BRACING AT ALL WALLS WITH UNSUPPORTED LENGTHS GREATER THAN 12'-0"

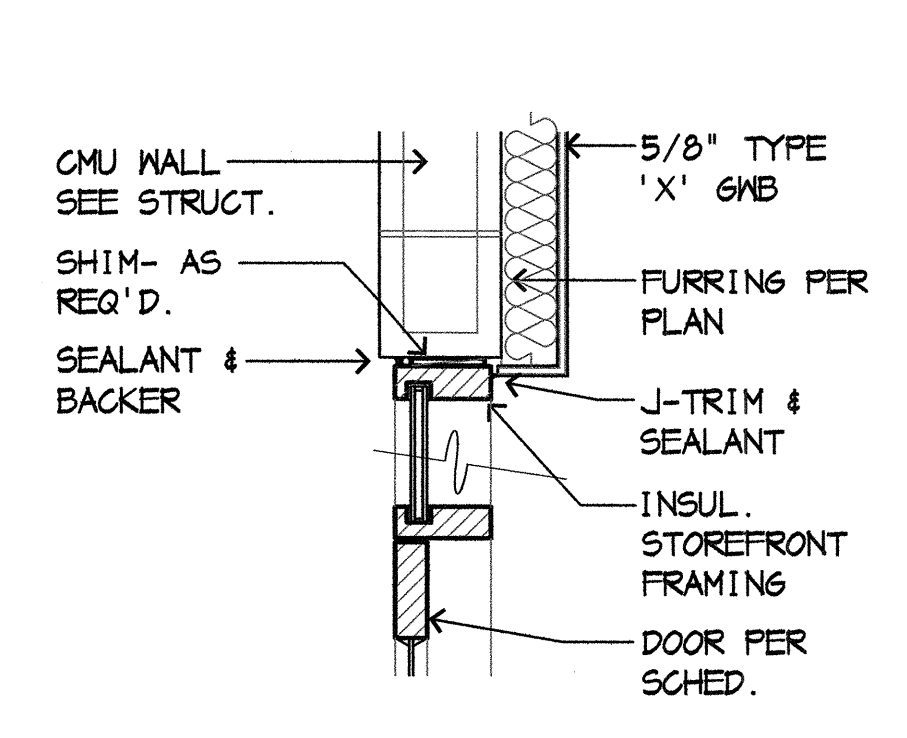
3 WALL BRACING  
1" = 1'-0"



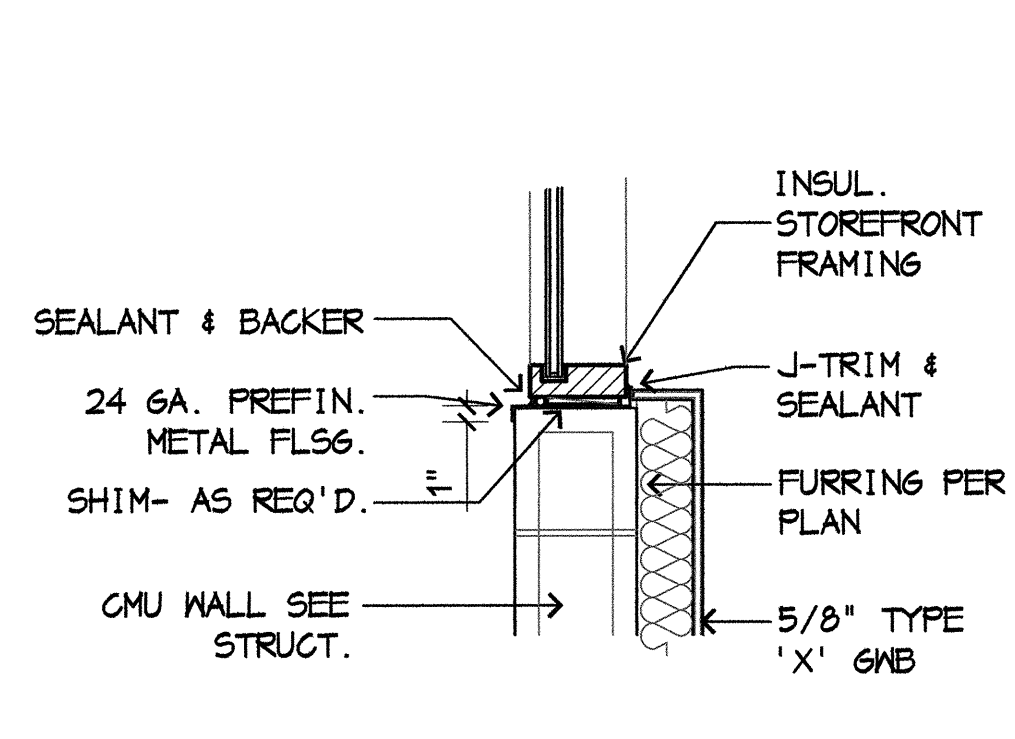
4 PERIMETER INSULATION  
SCALE: 3" = 1'-0"



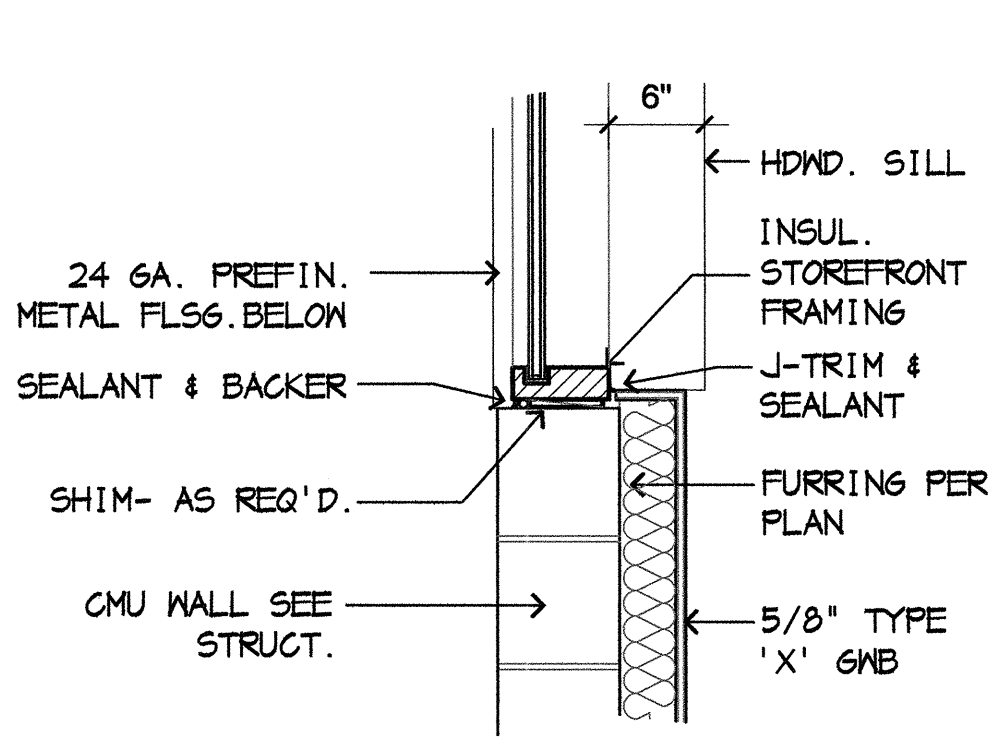
5 HANDRAIL  
3" = 1'-0"



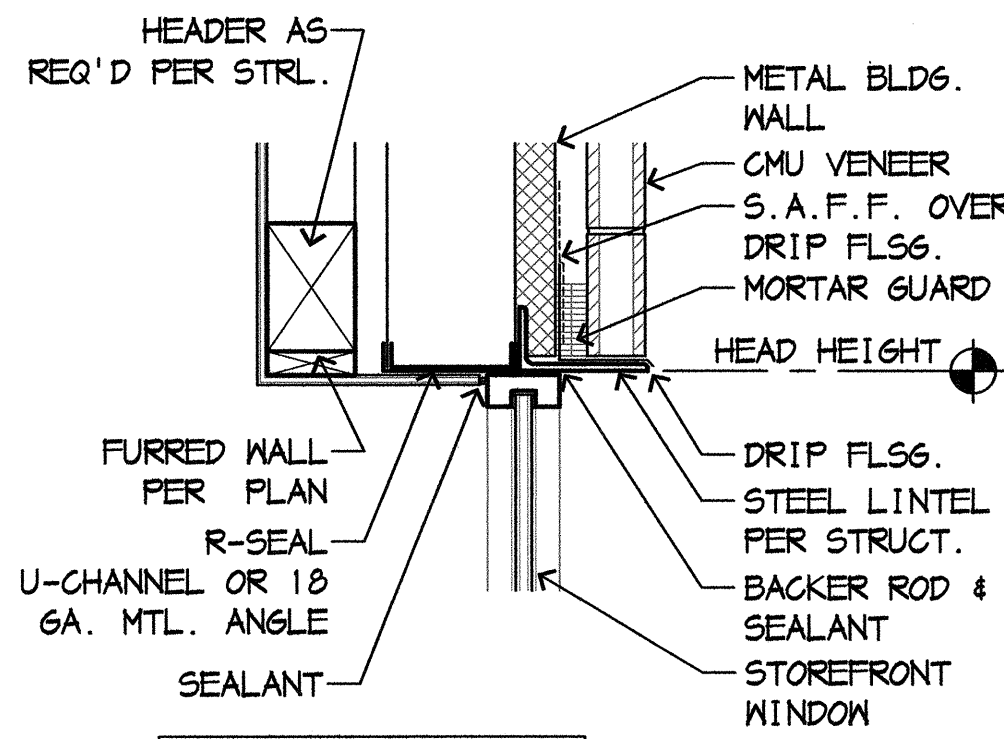
6 STOREFRONT HEAD  
1" = 1'-0"



7 STOREFRONT SILL  
1" = 1'-0"

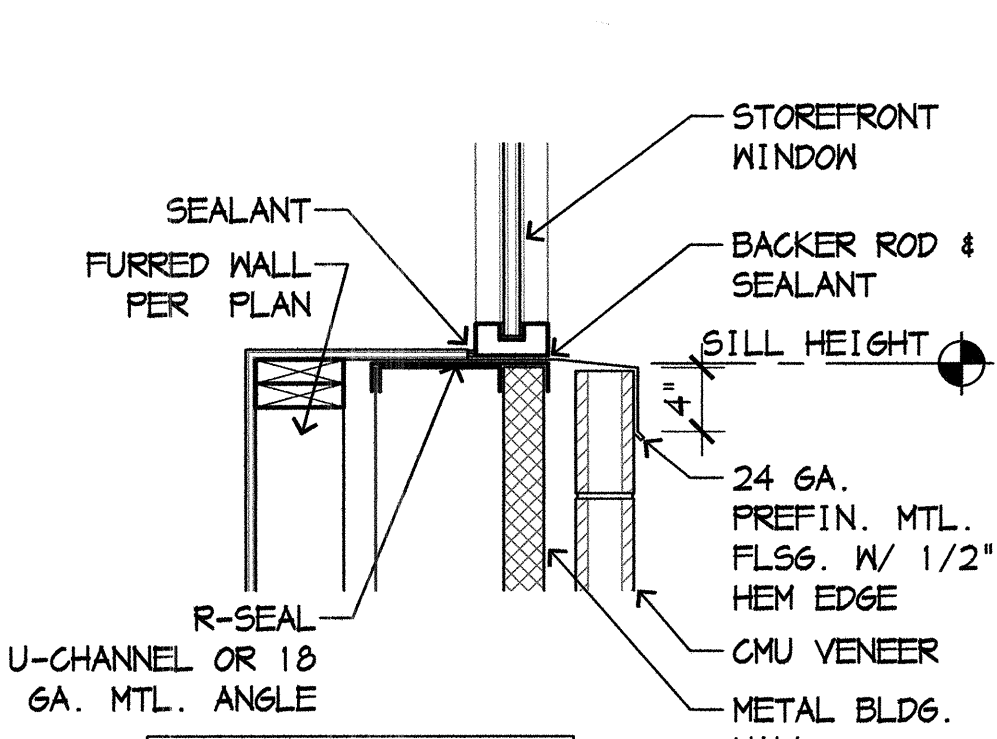


8 STOREFRONT JAMB  
1" = 1'-0"



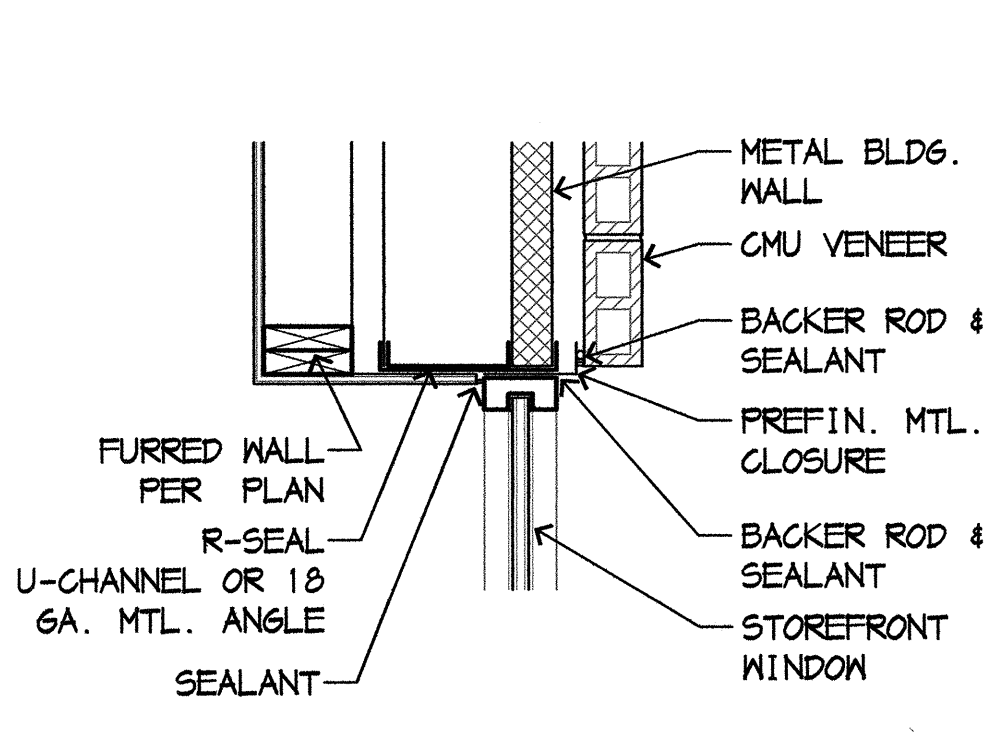
SEE DTL. 11 THIS DWG. FOR JAMB

9 STOREFRONT HEAD  
1" = 1'-0"

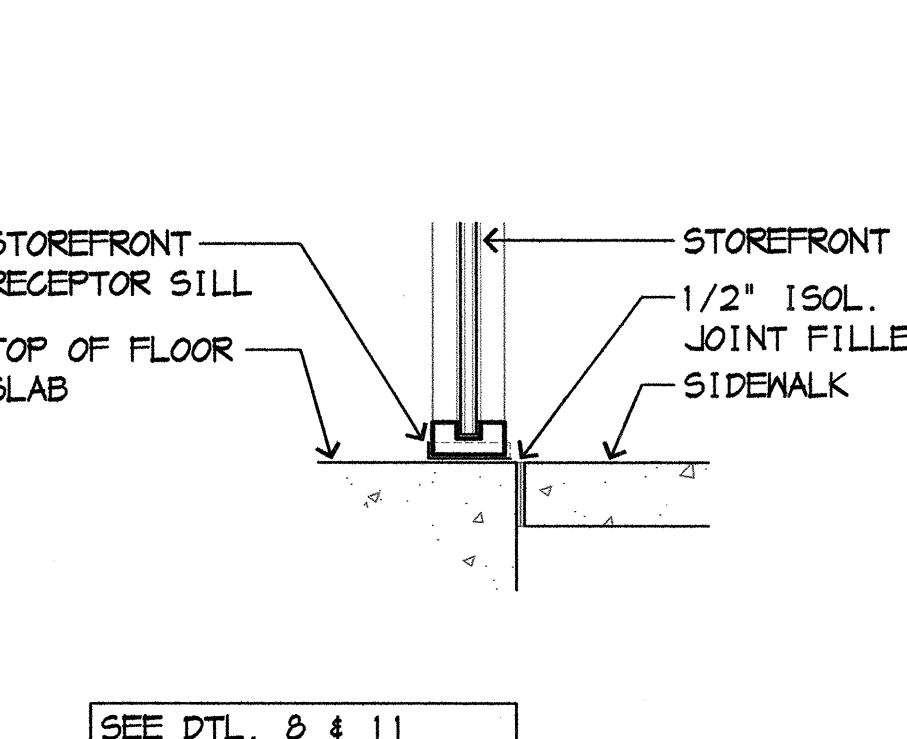


SEE DTL. 11 THIS DWG. FOR JAMB

10 STOREFRONT SILL  
1" = 1'-0"



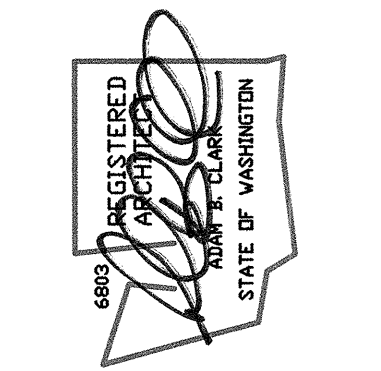
11 STOREFRONT JAMB  
1" = 1'-0"



SEE DTL. 8 & 11 THIS DWG. FOR JAMB

12 SF SILL @ FOUNDATION  
1" = 1'-0"

Date:	For:
02-11-19	PRELIMINARY
11-15-19	PRELIMINARY DESIGN REV'S
7-22-20	BID SET



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2017  
ARCHITECTURE

Tenant Improvements for:  
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Arlington, Washington

GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans)

CRITERIA

1. ALL MATERIALS, WORKMANSHIP, DESIGN, AND CONSTRUCTION SHALL CONFORM TO THE DRAWINGS, SPECIFICATIONS, THE 2015 EDITION OF THE INTERNATIONAL BUILDING CODE (IBC),
2. DESIGN LOADING CRITERIA
- |                |  |
|----------------|--|
| ROOF SNOW LOAD | 25 PSF   |
| WIND :         | ANALYSIS PROCEDURE: ASCE 7-10 CHAPTER 27 "PART II - ENCLOSED SIMPLE DIAPHRAGM" RISK CATEGORY II 110 MPH EXPOSURE "B" TOPOGRAPHIC FACTOR Kzt = 1.0  |
| EARTHQUAKE :   | ANALYSIS PROCEDURE: IBC "EQUIVALENT LATERAL FORCE PROCEDURE" SEISMIC DESIGN CATEGORY (SDC) = D RISK CATEGORY = II SEISMIC SITE CLASS = D IMPORTANCE FACTOR Ie = 1.0 MAPPED MCE Ss = 1.37; S1 = 0.53 DESIGN ACCELERATION Sds = 0.91; Sd1 = 0.53 SEISMIC RESISTING SYSTEM: ORDINARY REINFORCED CMU WALL, R = 2 |

SEE PLANS FOR ADDITIONAL LOADING CRITERIA.

3. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH ARCHITECTURAL DRAWINGS FOR BIDDING AND CONSTRUCTION. CONTRACTOR SHALL VERIFY DIMENSIONS AND CONDITIONS FOR COMPATIBILITY AND SHALL NOTIFY ARCHITECT OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION.
4. CONTRACTOR SHALL VERIFY ALL EXISTING DIMENSIONS, MEMBER SIZES, AND CONDITIONS PRIOR TO COMMENCING ANY WORK. ALL DIMENSIONS OF EXISTING CONSTRUCTION SHOWN ON THE DRAWINGS ARE INTENDED AS GUIDELINES ONLY AND MUST BE VERIFIED.
5. CONTRACTOR SHALL PROVIDE TEMPORARY BRACING FOR THE STRUCTURE AND STRUCTURAL COMPONENTS UNTIL ALL FINAL CONNECTIONS HAVE BEEN COMPLETED IN ACCORDANCE WITH THE PLANS.
6. CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SAFETY PRECAUTIONS AND THE METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES REQUIRED TO PERFORM THEIR WORK. THE STRUCTURAL ENGINEER HAS NO OVERALL SUPERVISORY AUTHORITY OR ACTUAL AND/OR DIRECT RESPONSIBILITY FOR THE SPECIFIC WORKING CONDITIONS AT THE SITE AND/OR FOR ANY HAZARDS RESULTING FROM THE ACTIONS OF ANY TRADE CONTRACTOR. THE STRUCTURAL ENGINEER HAS NO DUTY TO INSPECT, SUPERVISE, NOTE, CORRECT, OR REPORT ANY HEALTH OR SAFETY DEFICIENCIES OF THE OWNER, CONTRACTORS, OR OTHER ENTITIES OR PERSONS AT THE PROJECT SITE.
7. CONTRACTOR-INITIATED CHANGES SHALL BE SUBMITTED IN WRITING TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR APPROVAL PRIOR TO FABRICATION OR CONSTRUCTION. CHANGES SHOWN ON SHOP DRAWINGS ONLY WILL NOT SATISFY THIS REQUIREMENT.
8. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, SIMILAR DETAILS OF CONSTRUCTION SHALL BE USED, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. WHERE INFORMATION ON THE DRAWINGS IS IN CONFLICT WITH THE SPECIFICATIONS, THE MORE STRINGENT SHALL APPLY, SUBJECT TO REVIEW AND APPROVAL BY THE ARCHITECT AND THE STRUCTURAL ENGINEER. DO NOT SCALE THE DRAWINGS.
9. ALL STRUCTURAL SYSTEMS WHICH ARE COMPOSED OF FIELD ERECTED COMPONENTS SHALL BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE AND ERECTION IN ACCORDANCE WITH INSTRUCTIONS PREPARED BY THE SUPPLIER.
10. SHOP DRAWINGS FOR REINFORCING STEEL (FOR BOTH CONCRETE AND MASONRY CONSTRUCTION), STRUCTURAL STEEL, STEEL ROOF TRUSSES, AND ROOF METAL DECKING, SHALL BE SUBMITTED TO THE ARCHITECT AND STRUCTURAL ENGINEER FOR REVIEW PRIOR TO FABRICATION OF THESE ITEMS.
11. SHOP DRAWING REVIEW: DIMENSIONS AND QUANTITIES ARE NOT REVIEWED BY THE ENGINEER OF RECORD, AND THEREFORE MUST BE VERIFIED BY THE CONTRACTOR. CONTRACTOR SHALL REVIEW AND STAMP DRAWINGS PRIOR TO REVIEW BY ENGINEER OF RECORD. CONTRACTOR SHALL REVIEW DRAWINGS FOR CONFORMANCE WITH THE MEANS, METHODS, TECHNIQUES, SEQUENCES AND OPERATIONS OF CONSTRUCTION, AND ALL SAFETY PRECAUTIONS AND PROGRAMS INCIDENTAL THERETO. A MINIMUM OF TWO WEEKS SHALL BE ALLOWED FOR REVIEW.
12. SPECIAL INSPECTION: MASONRY CONSTRUCTION, STRUCTURAL STEEL FABRICATION AND ERECTION (INCLUDING FIELD WELDING AND HIGH-STRENGTH FIELD BOLTING), ROOF METAL DECK INSTALLATION, EXPANSION BOLTS AND THREADED EXPANSION INSERTS, AND EPOXY GROUTED INSTALLATIONS, SHALL BE SUPERVISED IN ACCORDANCE WITH IBC SECTIONS 1704 & 1705 AND THE PROJECT SPECIFICATIONS BY A QUALIFIED TESTING AGENCY DESIGNATED BY THE OWNER. THE TESTING AGENCY SHALL SEND COPIES OF ALL STRUCTURAL TESTING AND INSPECTION REPORTS DIRECTLY TO THE OWNER, ARCHITECT, STRUCTURAL ENGINEER, CONTRACTOR AND BUILDING OFFICIAL. ANY MATERIALS WHICH FAIL TO MEET PROJECT SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT.

GEOTECHNICAL

13. FOUNDATION NOTES: ALLOWABLE SOIL PRESSURE AND LATERAL EARTH PRESSURE ARE ASSUMED AND THEREFORE MUST BE VERIFIED IN THE FIELD. IF SOILS ARE FOUND TO BE OTHER THAN ASSUMED, NOTIFY THE STRUCTURAL ENGINEER FOR POSSIBLE FOUNDATION REDESIGN.
- FOOTINGS SHALL BEAR ON FIRM, UNDISTURBED EARTH (CONTROLLED, COMPACTED STRUCTURAL FILL OR BOTH) AT LEAST 18" BELOW LOWEST ADJACENT FINISHED GRADE. FOOTING DEPTHS/ELEVATIONS SHOWN ON PLANS (OR IN DETAILS) ARE MINIMUM AND FOR GUIDANCE ONLY; THE ACTUAL ELEVATIONS OF FOOTINGS MUST BE ESTABLISHED BY THE CONTRACTOR IN THE FIELD. UNLESS OTHERWISE NOTED, FOOTINGS SHALL BE CENTERED UNDER COLUMNS OR WALLS ABOVE.

BACKFILL BEHIND ALL RETAINING WALLS WITH FREE DRAINING, GRANULAR FILL AND PROVIDE FOR SUBSURFACE DRAINAGE.

THE STRUCTURAL DESIGN IS BASED ON THE FOLLOWING ASSUMED VALUES:

ALLOWABLE SOIL PRESSURE	1500 PSF
PASSIVE SOIL PRESSURE	350 PCF
SOIL COEFFICIENT OF FRICTION	0.35
SOIL DENSITY	120 PCF

RENOVATION

14. DEMOLITION: VERIFY ALL EXISTING CONDITIONS BEFORE COMMENCING ANY DEMOLITION. SHORING SHALL BE INSTALLED TO SUPPORT EXISTING CONSTRUCTION AS REQUIRED AND IN A MANNER SUITABLE TO THE WORK SEQUENCES. EXISTING REINFORCING SHALL BE SAVED WHERE AND AS NOTED ON THE PLANS. SAW CUTTING, IF AND WHERE USED, SHALL NOT CUT EXISTING REINFORCING THAT IS TO BE SAVED. DEMOLITION DEBRIS SHALL NOT BE ALLOWED TO DAMAGE OR OVERLOAD THE EXISTING STRUCTURE. LIMIT CONSTRUCTION LOADING (INCLUDING DEMOLITION DEBRIS) ON EXISTING FLOOR SYSTEMS TO 40 PSF.

- A. ALL NEW OPENINGS THROUGH EXISTING WALLS, SLABS AND BEAMS SHALL BE ACCOMPLISHED BY SAW CUTTING WHEREVER POSSIBLE.
- B. VERIFY ALL EXISTING CONDITIONS AND LOCATION OF MEMBERS PRIOR TO CUTTING ANY OPENINGS.
- C. SMALL ROUND OPENINGS SHALL BE ACCOMPLISHED BY CORE DRILLING, IF POSSIBLE.
- D. WHERE NEW REINFORCING TERMINATES AT EXISTING CONCRETE, REBAR DOWELS EPOXIED INTO THE EXISTING CONCRETE SHALL BE PROVIDED TO MATCH HORIZONTAL REINFORCING, UNLESS OTHERWISE NOTED ON PLANS.

CONCRETE

17. CONCRETE SHALL BE MIXED, PROPORTIONED, CONVEYED AND PLACED IN ACCORDANCE WITH ACI 301. CONSTRUCTION TOLERANCES SHALL NOT EXCEED THOSE LISTED IN ACI 117. CONCRETE SHALL ATTAIN A 28-DAY STRENGTH OF Fc = 2500 PSI AND MIX SHALL CONTAIN NOT LESS THAN 5-1/2 SACKS OF CEMENT PER CUBIC YARD AND SHALL BE PROPORTIONED TO PRODUCE A SLUMP OF 5" OR LESS (BEFORE THE ADDITION OF ADMIXTURES). THE WATER/CEMENT RATIO SHALL NOT EXCEED 0.55 FOR FOOTINGS AND 0.45 FOR ALL SLABS AND EXPOSED CONCRETE UNLESS OTHERWISE NOTED. EXCEPT FOR FOOTINGS AND SLAB ON GRADE, AGGREGATE SIZE SHALL NOT EXCEED 3/4".

THE MINIMUM AMOUNT OF CEMENT AND THE MAXIMUM SLUMP MAY BE CHANGED IF A CONCRETE PERFORMANCE MIX IS SUBMITTED TO THE STRUCTURAL ENGINEER AND THE BUILDING DEPARTMENT FOR APPROVAL TWO WEEKS PRIOR TO PLACING ANY CONCRETE. (THE W/C RATIO LIMITS STILL APPLY). THE PERFORMANCE MIX SHALL INCLUDE THE AMOUNTS OF CEMENT, CEMENTITIOUS MATERIAL, FINE AND COARSE AGGREGATE, WATER AND ADMIXTURES AS WELL AS THE WATER CEMENT RATIO, SLUMP, CONCRETE YIELD AND SUBSTANTIATING STRENGTH DATA IN ACCORDANCE WITH ACI 301. CHEMICAL ADMIXTURES AND FLY ASH SHALL CONFORM TO ASTM C494 AND C618 RESPECTIVELY. FLY ASH PERCENTAGE OF TOTAL CEMENTITIOUS MATERIAL SHALL NOT EXCEED 20%. THE USE OF A PERFORMANCE MIX REQUIRES BATCH PLANT INSPECTION, THE COST OF WHICH SHALL BE BROUGHT TO THE ATTENTION OF THE OWNER. REVIEW OF MIX SUBMITTALS BY THE ENGINEER OF RECORD INDICATES ONLY THAT INFORMATION PRESENTED CONFORMS GENERALLY TO CONTRACT DOCUMENTS. CONTRACTOR MAINTAINS FULL RESPONSIBILITY FOR SPECIFIED PERFORMANCE.

ALL CONCRETE WITH SURFACES EXPOSED TO STANDING WATER SHALL BE AIR-ENTRAINED WITH AN AIR-ENTRAINING AGENT CONFORMING TO ASTM C260. TOTAL AIR CONTENT FOR FROST-RESISTANT CONCRETE SHALL BE IN ACCORDANCE WITH ACI 318-14 TABLE 19.3.3.1. ALL CONCRETE EXPOSED TO THE WEATHER AND SLABS-ON-GRADE SHALL OBTAIN A 28-DAY STRENGTH Fc OF 3,000 PSI IN ACCORDANCE WITH ACI 318 TABLE 19.3.2.1 AND IBC SECTION 1904.1. THIS INCREASE IN REQUIRED STRENGTH IS FOR DURABILITY ONLY (SPECIAL INSPECTION IS NOT REQUIRED). ALL CONCRETE TO RECEIVE A STEEL TROWELED FINISH SHALL NOT BE AIR-ENTRAINED.

18. REINFORCING STEEL SHALL CONFORM TO ASTM A615 (INCLUDING SUPPLEMENT S1), GRADE 60, fy = 60,000 PSI. EXCEPTIONS: ANY BARS SPECIFICALLY SO NOTED ON THE DRAWINGS AS GRADE 40 SHALL HAVE fy = 40,000 PSI. GRADE 60 REINFORCING STEEL INDICATED ON DRAWINGS TO BE WELDED SHALL CONFORM TO ASTM A706. REINFORCING STEEL COMPLYING WITH ASTM A615 (S1) MAY BE WELDED ONLY IF MATERIAL PROPERTY REPORTS INDICATING CONFORMANCE WITH WELDING PROCEDURES SPECIFIED IN A.W.S. D1.4 ARE SUBMITTED.
19. REINFORCING STEEL SHALL BE DETAILED (INCLUDING HOOKS AND BENDS) IN ACCORDANCE WITH ACI 315 AND 318. LAP ALL CONTINUOUS REINFORCEMENT #5 AND SMALLER 60 BAR DIAMETERS, 2'-0" MINIMUM. PROVIDE CORNER BARS AT ALL WALL AND FOOTING INTERSECTIONS. LAP CORNER BARS #5 AND SMALLER 60 BAR DIAMETERS OR 2'-0" MINIMUM. LAPS OF LARGER BARS SHALL BE MADE IN ACCORDANCE WITH ACI 318, CLASS B. PROVIDE (2) #4 MIN. U.N.O. TRIM BARS AROUND ALL OPENINGS IN CONCRETE WALLS OR SLABS EXTENDING 2'-6" PAST CORNERS, TYPICAL.
- NO BARS PARTIALLY EMBEDDED IN HARDENED CONCRETE SHALL BE FIELD BENT UNLESS SPECIFICALLY SO DETAILED OR APPROVED BY THE STRUCTURAL ENGINEER. NO REINFORCING BARS SHALL BE "WET-SET" INTO THE CONCRETE.

20. CONCRETE PROTECTION (COVER) FOR REINFORCING STEEL SHALL BE AS FOLLOWS:

FOOTINGS AND OTHER UNFORMED SURFACES CAST AGAINST EARTH	3"
FORMED SURFACES EXPOSED TO EARTH (i.e. WALLS BELOW GROUND) OR WEATHER	2"
SLABS AND WALLS (INTERIOR FACE)	1"

21. NON-SHRINK GROUT SHALL BE NON-METALLIC CONFORMING TO ASTM C1107 AND BE FURNISHED BY AN APPROVED MANUFACTURER AND SHALL BE MIXED AND PLACED IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. GROUT STRENGTH SHALL BE AT LEAST EQUAL TO THE MATERIAL ON WHICH IT IS PLACED (5000 PSI MINIMUM).

ANCHORAGE

22. EXPANSION BOLTS INTO CONCRETE SHALL BE "KNIK BOLT TZ" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1917 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

23. EXPANSION BOLTS INTO GROUT FILLED CMU SHALL BE "KNIK BOLT 3" EXPANSION ANCHORS AS MANUFACTURED BY HILTI CORP. INSTALLED IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-1885 INCLUDING STANDARD EMBEDMENT REQUIREMENTS U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION IS REQUIRED FOR ALL EXPANSION BOLT INSTALLATION.

24. DRIVE PINS, SHOT PINS AND OTHER POWDER-ACTUATED FASTENERS SHALL BE LOW VELOCITY TYPE FASTENERS AS MANUFACTURED BY HILTI CORPORATION. WHEN CALLED FOR IN THE DRAWINGS, PROVIDE THE APPROPRIATE FASTENER AS NOTED IN THE TABLE BELOW FOR EACH GIVEN APPLICATION. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORTS NO. ESR-2269 FOR THE X-U FASTENERS AND ESR-2319 FOR THE X-CP FASTENERS. MINIMUM EMBEDMENT IN CONCRETE SHALL BE 1" UNLESS OTHERWISE NOTED. MAINTAIN AT LEAST 3" TO NEAREST CONCRETE EDGE AND 4" CENTER TO CENTER SPACING. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES.

ALLOWABLE APPLICATION	ALLOWABLE FASTENER TYPE	SHEAR CAPACITY (LBS)	TENSION CAPACITY (LBS)
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO CONCRETE (2000 PSI MIN.)	X-U 21 P8 S15	190	165
LIGHT GAUGE STEEL 43 & 33 MILS (18 & 20 GA.) TO STRUCTURAL STEEL (3/16" MIN. TO 1/16" MAX)	X-U 19 P8 TH	445	360
LIGHT GAUGE STEEL 33 MILS (20 GA.) MIN. TO GROUTED CMU	X-U 32 P8 S15	220	225

25. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO CONCRETE SHALL BE INSTALLED USING "HIT-HY 200" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-3187, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

26. EPOXY-GROUTED ITEMS (THREADED RODS OR REINFORCING BAR) INTO GROUT FILLED CMU SHALL BE INSTALLED USING "HIT HY 210" AS MANUFACTURED BY HILTI CORP. INSTALL IN STRICT ACCORDANCE WITH I.C.C. REPORT NO. ESR-4143, INCLUDING STANDARD EMBEDMENT REQUIREMENTS, U.O.N. PROPOSED SUBSTITUTIONS SHALL BE SUBMITTED FOR REVIEW WITH I.C.C. OR IAPMO UES REPORTS INDICATING EQUIVALENT OR GREATER LOAD CAPACITIES. SPECIAL INSPECTION OF INSTALLATION IS REQUIRED.

MASONRY

27. CONCRETE MASONRY UNIT WALLS SHALL BE CONSTRUCTED OF MEDIUM WEIGHT UNITS CONFORMING TO ASTM C90, LAID IN A RUNNING BOND. CONTACT ENGINEER FOR RE-DESIGN OF REINFORCING WHERE STACK BOND LAYOUT IS REQUIRED. LINEAR SHRINKAGE SHALL NOT EXCEED 0.065%. MORTAR SHALL BE TYPE "S" IN ACCORDANCE WITH ASTM C270. GROUT SHALL CONFORM TO IBC REQUIREMENTS AND ATTAIN A MINIMUM COMPRESSIVE STRENGTH OF 2,000 PSI AT 28 DAYS, DESIGN F'm = 2,000 PSI. FULL STRESSES ARE REQUIRED. STRENGTH SHALL BE VERIFIED BY THE UNIT STRENGTH METHOD IN ACCORDANCE WITH TMS 602-13.

UNLESS NOTED OTHERWISE, PROVIDE THE FOLLOWING REINFORCEMENT:

WALL THICKNESS	VERTICAL BARS	HORIZONTAL BARS
8" WALLS	#4 @ 24" O.C.	(2) #4 @ 48" O.C.

IN ADDITION, PROVIDE (2) #4 VERT. AT EACH SIDE OF OPENINGS, AT WALL CORNERS AND INTERSECTIONS AND AT FREE ENDS OF WALLS AND (2) #4 HORIZ. AT ELEVATED ROOF LEVELS, AT TOPS OF WALLS AND ABOVE AND BELOW ALL OPENINGS. ALL HORIZONTAL REINFORCEMENT SHALL BE PLACED IN BOND BEAMS. EXTEND REINFORCEMENT AROUND OPENINGS 2'-0" BEYOND FACE OF OPENING. IF 2'-0" IS UNAVAILABLE, EXTEND AS FAR AS POSSIBLE AND HOOK. PROVIDE CORNER BARS TO LAP HORIZONTAL REINFORCING AT CORNERS AND INTERSECTIONS. UNLESS NOTED OTHERWISE, LAP ALL REINFORCING STEEL IN CMU 48 BAR DIAMETERS, 2'-0" MINIMUM.

ALL CELLS ARE TO BE SOLID GROUTED. ALL REINFORCEMENT SHALL BE IN PLACE PRIOR TO GROUTING AND SHALL BE HELD AT TOP, BOTTOM AND 1/2 BAR DIAMETERS (MAX.) O.C. PER ACI 530.1, GROUT POURS SHALL NOT EXCEED 5'-4" IN HEIGHT UNLESS A TEST PANEL IS CONSTRUCTED BY THE MASON AND APPROVED BY THE STRUCTURAL ENGINEER. PROVIDE CLEANOUT HOLES AT BOTTOM OF ALL CELLS CONTAINING REINFORCEMENT FOR ALL GROUT POURS OVER 5'-4" IN HEIGHT. PROVIDE 1 1/2 IN. GROUT KEYS BETWEEN EACH POUR.

STEEL

28. STRUCTURAL STEEL DESIGN, FABRICATION AND ERECTION SHALL BE BASED ON THE LATEST EDITIONS OF THE A.I.S.C. SPECIFICATIONS AND CODES:

- A. AISC - STEEL CONSTRUCTION MANUAL, 14<sup>TH</sup> EDITION
- B. CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES.
- C. RSCC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS.

For:	PERMIT SET	BID SET			
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GENERAL STRUCTURAL NOTES

Drawing:	S1.0
Job Number:	20046.02



GENERAL STRUCTURAL NOTES

(The following apply unless shown otherwise on the plans.)

30. STRUCTURAL STEEL, WIDE FLANGE (W AND WT) SHAPES SHALL CONFORM TO ASTM A992, Fy = 50 KSI; ALL OTHER ROLLED SHAPES SHALL CONFORM TO ASTM A36, Fy = 36 KSI. STEEL PLATE SHALL CONFORM TO ASTM A36, Fy = 36 KSI. STEEL PIPE SHALL CONFORM TO ASTM A53, TYPE E OR S, GRADE B, Fy = 35 KSI. STRUCTURAL TUBING SHALL CONFORM TO ASTM A500, GRADE B, Fy = 46 KSI. CONNECTION BOLTS SHALL CONFORM TO ASTM A325. ANCHOR BOLTS SHALL CONFORM TO ASTM F1554 GRADE 36, Fy = 36 KSI.

STEEL BEAMS ARE EQUALLY SPACED BETWEEN DIMENSIONED POINTS. ALL STEEL ANCHORS AND TIES AND OTHER MEMBERS EMBEDDED IN CONCRETE OR MASONRY SHALL BE LEFT UNPAINTED. ALL STEEL TO BE FIREPROOFED SHALL BE LEFT UNPAINTED. ALL OTHER STEEL SHALL HAVE ONE COAT OF APPROVED SHOP PAINT.

STRUCTURAL STEEL AND CONNECTIONS EXPOSED TO WEATHER OR EARTH SHALL BE HOT-DIPPED GALVANIZED AFTER FABRICATION IN COMPLIANCE WITH ASTM A123. GALVANIZE BOLTS AND SIMILAR THREADED FASTENERS EXPOSED TO WEATHER OR EARTH IN ACCORDANCE WITH ASTM A153. ALL FIELD WELDS EXPOSED TO WEATHER OR EARTH SHALL BE COATED WITH BRUSH APPLIED ZINC RICH PAINT COMPLYING WITH ASTM A780 (Z.R.C. OR EQUIVALENT).

A MINIMUM OF TWO BOLTS ARE REQUIRED FOR ALL CONNECTIONS. ALTERNATE CONNECTIONS TO THOSE SHOWN ON THESE DRAWINGS WILL REQUIRE PRIOR APPROVAL OF THE ENGINEER.

ALL MEMBERS ARE TO BE ERECTED WITH THE NATURAL MILL CAMBER OR INDUCED CAMBER UP, UNLESS OTHERWISE NOTED ON THE DRAWINGS. BEAM CAMBER ON THE DRAWINGS IS THE UPWARD CAMBER REQUIRED IN THE BEAM AS DELIVERED TO THE JOBSITE. CONTRACTOR TO CONSIDER CAMBER LOSS, IF ANY, DUE TO SHIPPING AND HANDLING.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL ERECTION AIDS AND JOINT PREPARATIONS THAT INCLUDE, BUT ARE NOT LIMITED TO, ERECTION ANGLES, LIFT HOLES, AND OTHER AIDS, WELDING PROCEDURES, REQUIRED ROOT OPENINGS, ROOT FACE DIMENSIONS, GROOVE ANGLES, BACKING BARS, COPEs, SURFACE ROUGHNESS VALUES AND UNEQUAL PARTS.

31. ALL A325 CONNECTION BOLTS SHALL BE INSTALLED TO THE SNUG-TIGHT CONDITION PER RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING HIGH STRENGTH BOLTS IN STRICT ACCORDANCE WITH THE MANUFACTURER'S PUBLISHED RECOMMENDATIONS. ALL NUTS SHALL CONFORM TO ASTM A563. ALL WASHERS SHALL CONFORM TO ASTM F436 OR ASTM F454 TYPE 325. ALL BOLT HOLES SHALL BE STANDARD SIZE UNLESS OTHERWISE NOTED.

32. ALL WELDING SHALL BE IN CONFORMANCE WITH A.I.S.C. AND A.M.S. STANDARDS AND SHALL BE PERFORMED BY M.A.B.O. CERTIFIED WELDERS USING E70 XX ELECTRODES. ONLY PREQUALIFIED WELDS (AS DEFINED BY A.M.S.) SHALL BE USED. WELDING OF GRADE 60 REINFORCING BARS (IF REQUIRED) SHALL BE PERFORMED USING LOW HYDROGEN ELECTRODES. WELDING OF GRADE 40 REINFORCING BARS (IF REQUIRED) SHALL BE PERFORMED USING E70XX ELECTRODES. WELDING WITHIN 4" OF COLD BENDS IN REINFORCING STEEL IS NOT PERMITTED. SEE REINFORCING NOTE FOR MATERIAL REQUIREMENTS OF WELDED BARS. ALL WELDING OF STAINLESS STEEL SHALL USE E304 ELECTRODES WITH A SMAW PROCESS. ALL WELDING SHALL BE PERFORMED BY WELDERS WITH AWS / M.A.B.O. CERTIFICATION WITH THE MATERIAL AND METHOD REQUIRED.

SHOP DRAWINGS SHALL SHOW ALL WELDING WITH AWS A2.4 SYMBOLS. WELDS SHOWN ON DRAWINGS ARE MINIMUM SIZES. INCREASE WELD SIZE TO AWS MINIMUM SIZES BASED ON PLATE THICKNESS. MINIMUM WELDING SHALL BE 3/16-INCH. THE WELDS SHOWN ARE FOR THE FINAL CONNECTIONS. FIELD WELD ARROWS ARE SHOWN WHERE A FIELD WELD IS REQUIRED BY THE STRUCTURAL DESIGN; THE CONTRACTOR IS RESPONSIBLE FOR DETERMINING IF A WELD SHOULD BE SHOP OR FIELD WELDED IN ORDER TO FACILITATE THE STRUCTURAL STEEL DELIVERY AND ERECTION.

33. OPEN WEB STEEL JOISTS (INCLUDING BRIDGING) SHALL CONFORM TO IBC SECTION 2207 AND THE SPECIFICATIONS OF THE STEEL JOIST INSTITUTE (SJI), LATEST EDITION, FOR THE JOIST SERIES DESIGNATED ON THE PLANS AND THE LOADING BELOW (EXCEPT AS SHOWN ON THE LOAD DIAGRAMS DESIGNATED ON THE PLANS):

LIVE (SNOW) LOAD	25 PSF
DEAD LOAD	10 PSF
TOTAL LOAD	35 PSF

NET WIND UPLIFT (ENCLOSED BUILDINGS)	15 PSF
NET WIND UPLIFT (OPEN ROOF / OVERHANGS)	20 PSF

MAXIMUM TOTAL DEFLECTION SHALL BE LESS THAN OR EQUAL TO L/240 OF THE TOTAL SPAN AND MAXIMUM LIVE LOAD DEFLECTION SHALL BE LESS THAN OR EQUAL TO L/360 OF THE TOTAL SPAN. PROVIDE ADDITIONAL TRUSSES (AS REQUIRED) TO CARRY ALL CONCENTRATED LOADS AND MECHANICAL UNITS.

ENDS OF BRIDGING ROWS SHALL BE FIELD WELDED TO STRUCTURAL STEEL MEMBERS OR TO PLATES EMBEDDED IN CONCRETE OR MASONRY UNLESS DETAILED OTHERWISE. JOIST MANUFACTURER SHALL CHECK ROOF JOIST AND PROVIDE UPLIFT BRIDGING AS REQUIRED TO ADEQUATELY BRACE THE BOTTOM CHORD AGAINST LATERAL MOVEMENT UNDER WIND UPLIFT PRESSURES (SEE DESIGN CRITERIA NOTE FOR WIND CRITERIA). JOIST BOTTOM CHORD EXTENSIONS AND TOP CHORD ERECTION BOLT HOLES SHALL BE PROVIDED AT ALL COLUMNS TO MEET OSHA REQUIREMENTS.

THE JOIST MANUFACTURER SHALL BE A MEMBER OF THE SJI AND SHALL FURNISH TO THE BUILDING OFFICIAL A CERTIFICATE INDICATING COMPLIANCE WITH IBC SECTION 2207 AND WHICH ALSO IDENTIFIES THE JOISTS DELIVERED FOR THIS SPECIFIC PROJECT.

DETAILED DRAWINGS INDICATING CHORD AND WEB SIZES AND ALL CONNECTIONS SHALL BE SUBMITTED FOR EACH JOIST TYPE TO THE ENGINEER OF RECORD FOR REVIEW PRIOR TO FABRICATION.

34. JOIST MANUFACTURER'S NOTE: THE JOIST CONFIGURATIONS, INCLUDING SPACING, DEPTH AND MEMBER SIZES SHOWN ON THE DRAWINGS, INDICATE THE DESIRED JOIST CONFIGURATIONS AND ARE TO BE COMPLIED WITH WHEREVER POSSIBLE. IF A JOIST MANUFACTURER IS UNABLE TO MEET THE LOAD REQUIREMENTS SPECIFIED WITH THE TRUSS CONFIGURATION INDICATED, THE MANUFACTURER IS TO SUBMIT WRITTEN NOTICE TO THAT AFFECT TO THE ARCHITECT PRIOR TO SUBMITTING A COST PROPOSAL OR BID.

IF A DIFFERENT SYSTEM IS PROPOSED THAT REQUIRES REVISIONS TO PRESENT STRUCTURAL FRAMING OR DETAILS, SUCH SYSTEM SHALL BE CONSIDERED SUBJECT TO THE APPROVAL OF THE OWNER, ARCHITECT, AND STRUCTURAL ENGINEER.

IT IS THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND JOIST MANUFACTURER TO VERIFY THE WEIGHT AND LOCATIONS OF ALL MECHANICAL EQUIPMENT PRIOR TO SUBMITTING SHOP DRAWINGS. IT SHALL BE NOTED IN THE JOIST MANUFACTURER'S BID WHETHER OR NOT AN ALLOWANCE HAS BEEN MADE FOR MECHANICAL UNITS.

JOIST SHOP DRAWINGS WILL NOT BE REVIEWED WITHOUT CALCULATIONS STAMPED BY A STRUCTURAL ENGINEER LICENSED IN THE STATE OF WASHINGTON.

35. METAL ROOF DECKING - PROVIDE SIZE, TYPE, GAUGE, AND ATTACHMENT TO THE SUPPORTING STRUCTURE AS SHOWN ON THE PLANS. ALTERNATES MUST BE CONNECTED ACCORDING TO PUBLISHED I.C.C. OR IAPMO UES CRITERIA FOR DIAPHRAGM SHEARS SHOWN. PROVIDE SHORING WHERE REQUIRED PER MANUFACTURER'S PUBLISHED CRITERIA. ALL DECKING SHALL CONFORM TO THE REQUIREMENTS OF THE STEEL DECK INSTITUTE.

36. COLD-FORMED STEEL FRAMING NOTES - THE FOLLOWING APPLY UNLESS OTHERWISE SHOWN ON THE PLANS:

A. COLD-FORMED STEEL FRAMING MEMBERS SHALL BE OF THE SHAPE, SIZE, AND GAUGE SHOWN ON THE PLANS. ALL FRAMING MEMBERS SHALL COMPLY WITH I.C.C. REPORT NO. ESR-3064P. NOTATIONS ON THE DRAWINGS, RELATING TO MEMBER TYPES AND SIZES OR MISCELLANEOUS FRAMING ITEMS, REFER TO CATALOG NUMBERS OF MEMBERS MANUFACTURED BY THE STEEL STUD MANUFACTURERS ASSOCIATION (SSMA). PRODUCTS BY OTHER MANUFACTURERS MAY BE SUBSTITUTED FOR FRAMING SHOWN, PROVIDED THEY ARE EQUIVALENT IN SHAPE, SIZE, STIFFNESS, AND STRENGTH. ALTERNATE FRAMING SHALL BE SUBJECT TO REVIEW BY THE ARCHITECT AND STRUCTURAL ENGINEER PRIOR TO FABRICATION. ALL COLD-FORMED STEEL FRAMING SHALL CONFORM TO THE A.I.S.I. "SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."

B. MATERIAL:

METAL FRAMING SHALL BE GALVANIZED UNLESS OTHERWISE NOTED, CONFORMING AS FOLLOWS:

ASTM A653 S5 GRADE 50, CLASS 1 OR 3	Fy = 50 KSI	118, 97, 68, AND 54 MIL
ASTM A653 S5 GRADE 33	Fy = 33 KSI	43 AND 33 MIL

WHERE NOTED, PAINTED STUDS SHALL CONFORM TO:  
ASTM A1011 S5 GRADE 50 Fy = 50 KSI 118, 97, 68, AND 54 MIL

C. WELDING OF COLD-FORMED METAL FRAMING SHALL CONFORM TO AWS D13 AND SHALL BE PERFORMED BY WELDERS QUALIFIED TO PRODUCE THE SPECIFIED CLASSES OF WELD.

D. WALL FRAMING: ALL STUD WALLS SHOWN AND NOT OTHERWISE NOTED SHALL BE 400S162-43 @ 16" O.C. AT INTERIOR WALLS AND 600S162-43 AT 16" O.C. AT EXTERIOR WALLS. TWO STUDS MINIMUM SHALL BE PROVIDED AT THE END OF ALL WALLS AND AT EACH SIDE OF ALL OPENINGS. TWO 800S162-54 HEADERS SHALL BE PROVIDED OVER ALL OPENINGS UNLESS OTHERWISE NOTED. PROVIDE CONTINUOUS FULL WIDTH BLOCKING AT 1/3 POINTS OF ALL STUD WALLS UNLESS NOTED OTHERWISE. MAXIMUM GAP BETWEEN STUD AND TRACK AT ANY POINT SHALL NOT EXCEED 1/16-INCH. NO SPLICES ARE PERMITTED IN STUDS.

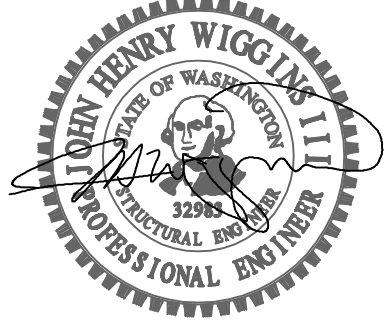
ALL STUD WALLS SHALL HAVE THEIR BOTTOM TRACKS ATTACHED TO CONCRETE WITH 5/32" DIAMETER DRIVE-PINS @ 16" O.C. UNLESS INDICATED OTHERWISE. INDIVIDUAL MEMBERS OF BUILT-UP POSTS SHALL BE WELDED OR SCREWED TO EACH OTHER IN ACCORDANCE WITH THE DETAILS. WHEN NOT OTHERWISE NOTED, PROVIDE GYPSUM WALLBOARD ON INTERIOR SURFACES AND GYPSUM SHEATHING ON EXTERIOR SURFACES SCREWED TO ALL STUDS, TOP AND BOTTOM TRACKS, AND BLOCKING WITH SCREWS AT 12" O.C. ALL SCREWS SHALL BE "GRABBER" TYPE FASTENERS COMPLYING WITH I.C.C. REPORT NO. ESR-1271. ALL SPECIFIED PNEUMATIC FASTENERS SHALL BE ET&F, COMPLYING WITH I.C.C. REPORT NO. ESR-1777.

TRACK SECTIONS SHALL BE UNPUNCHED AND HAVE AT LEAST 1-1/2" FLANGES AND MATCH STUD THICKNESS.

WALLS WHICH HAVE SHEATHING CONNECTED ON ONE SIDE ONLY SHALL HAVE UNSHEATHED FLANGES Laterally SUPPORTED IN ACCORDANCE WITH THE DETAILS.

ABBREVIATIONS			
@	At	L	Angle
d	Penny (Nails)	LB.	Pound
φ	Diameter	LL	Live Load
°	Degrees	LLH	Long Leg Horizontal
...#	Pounds	LLV	Long Leg Vertical
#...	Number	LONGIT.	Longitudinal
		LT. WT.	Lightweight
(A)	Above		
A.B.	Anchor Bolt	MAX.	Maximum
ADD'L	Additional	MECH.	Mechanical
ALT.	Alternate	MEZZ.	Mezzanine
APPROX.	Approximate	MF	Moment Frame
ARCH.	Architect	MFR.	Manufacturer
		MIN.	Minimum
(B)	Below	MISC.	Miscellaneous
B/	Bottom of	MK.	Mark
BF	Braced Frame		
BLKG.	Blocking	(N)	New
BLDG.	Building	N.	North
BM.	Beam	N.S.	Near Side
BOT.	Bottom	NOM.	Nominal
BRG.	Bearing	NTS	Not to Scale
BTWN.	Between		
		O.C.	On Center
CL	Centerline	O.D.	Outside Diameter
C	Camber	O.F.	Outside Face
CIP	Cast in Place	O.H.	Overhang
C-J.	Construction Joint or Control Joint	OPNG.	Opening
CJP	Complete Joint Penetration	OPP.	Opposite
CLG.	Ceiling	ONSJ	Open Web Steel Joist
CLR.	Clear		
CMU	Concrete Masonry Unit	PAF	Powder Actuated Fastener
COL.	Column	PC	Precast
CONC.	Concrete	PERM.	Permanent
CONN.	Connections	PERP.	Perpendicular
CONGT.	Construction	PJP	Partial Joint Penetration
CONT.	Continuous	PL or PL	Plate
CSK.	Countersink	PLF	Pounds per linear Foot
		PLYWD	Plywood
DBA	Deformed Bar Anchor	PREFAB.	Prefabricated
DBL.	Double	PSF	Pounds per Square Foot
DEG.	Degree	PSI	Pounds per Square Inch
DF	Doug Fir-Larch	P.T. or PT	Post-Tensioning
DIA.	Diameter	P/T	Pressure-Treated
DIA8.	Diagonal		
DIAPH.	Diaphragm	RAD.	Radius
DIM.	Dimension	REF.	Reference
DN.	Down	REINF.	Reinforce or Reinforcement
DO	Ditto	REQD.	Required
DTL.	Detail	REV.	Revise
DWG.	Drawing	R.O.	Rough Opening
(E)	Existing	S.	South
E.	East	SCH. or SCHED.	Schedule
EA.	Each	SECT.	Section
E.F.	Each Face	SHT.	Sheet
EL.	Elevation	SIM.	Similar
ELEV.	Elevator	SO6.	Slab On Grade
EMBED.	Embedment	SPEC.	Specification
ENGR.	Engineer	SQ.	Square
EQ.	Equal	SQ. FT.	Square Feet
E.W.	Each Way	SQ. IN.	Square Inch(es)
EXP.	Expansion	SPP	Spruce-Pine-Fir
EXT.	Exterior	S.S.	Stainless Steel
		STD.	Standard
FDN.	Foundation	STIFF.	Stiffener
FIN.	Finish	STL.	Steel
FLR.	Floor	STR.	Structural
FRP	Fiber Reinforced Polymer	SUB.	Substitute
F.S.	Far Side	SYM.	Symmetrical
FT.	Foot or Feet		
FTG.	Footing	T/	Top of
		T&B	Top and Bottom
GA.	Gauge	T&G	Tongue & Groove
GAL.V.	Galvanized	TEMP.	Temporary
GL	Glue Laminated	THRU	Through
GWB	Gypsum Wall Board	T.O.C.	Top of Concrete
		T.O.S.	Top of Steel
HDG	Hot Dipped Galvanized	T.O.W.	Top of Wall
HF	Hem Fir	TRANS.	Transverse
HGR.	Hanger	TS	Tube Steel
HORIZ.	Horizontal	TYP.	Typical
HSS	Hollow Structural Section		
HT.	Height	U.O.N.	Unless Otherwise Noted
I.D.	Inside Diameter	VERT.	Vertical
I.F.	Inside Face	VIF	Verify in Field
IN.	Inch		
INFO.	Information	W.	West
INT.	Interior	W/ or w/	With
		W.H.S.	Welded Headed Stud
JT.	Joint	W/O	Without
		WP	Work Point
K	Kips	W.T.S.	Welded Threaded Stud
KSF	Kips per Square Foot	WNF	Welded Wire Fabric
KSI	Kips per Square Inch		
		X SECT.	Cross Section
		X-STR	Extra Strong
		XX-STR	Double Extra Strong

For:	PERMIT SET				
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Date:	4/22/20	7/22/20			



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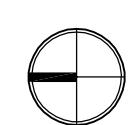
2017  
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Tenant Improvements for:  
Stiliaguamish Tribal Courthouse

17014 58th Avenue NE  
Arlington, Washington

GENERAL STRUCTURAL NOTES

Drawing:	S1.1
Job Number:	
	20046.02



SCALE: 1/8" = 1'-0"

INDICATES (E) FOOTING

INDICATES (E) FOUNDATION WALL OR (E) CMU WALL ABOVE FLOOR FRAMING

INDICATES FOOTING

INDICATES 8" CMU WALL. SEE GENERAL NOTES AND DETAILS FOR REINFORCING REQUIREMENTS

INDICATES LIGHT GAUGE STEEL BEARING OR SHEAR WALL AT THIS LEVEL. SEE PLAN NOTES 5 & 6

INDICATES (E) LIGHT GAUGE STEEL WALL AT THIS LEVEL.

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Tenant Improvements for:

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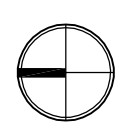
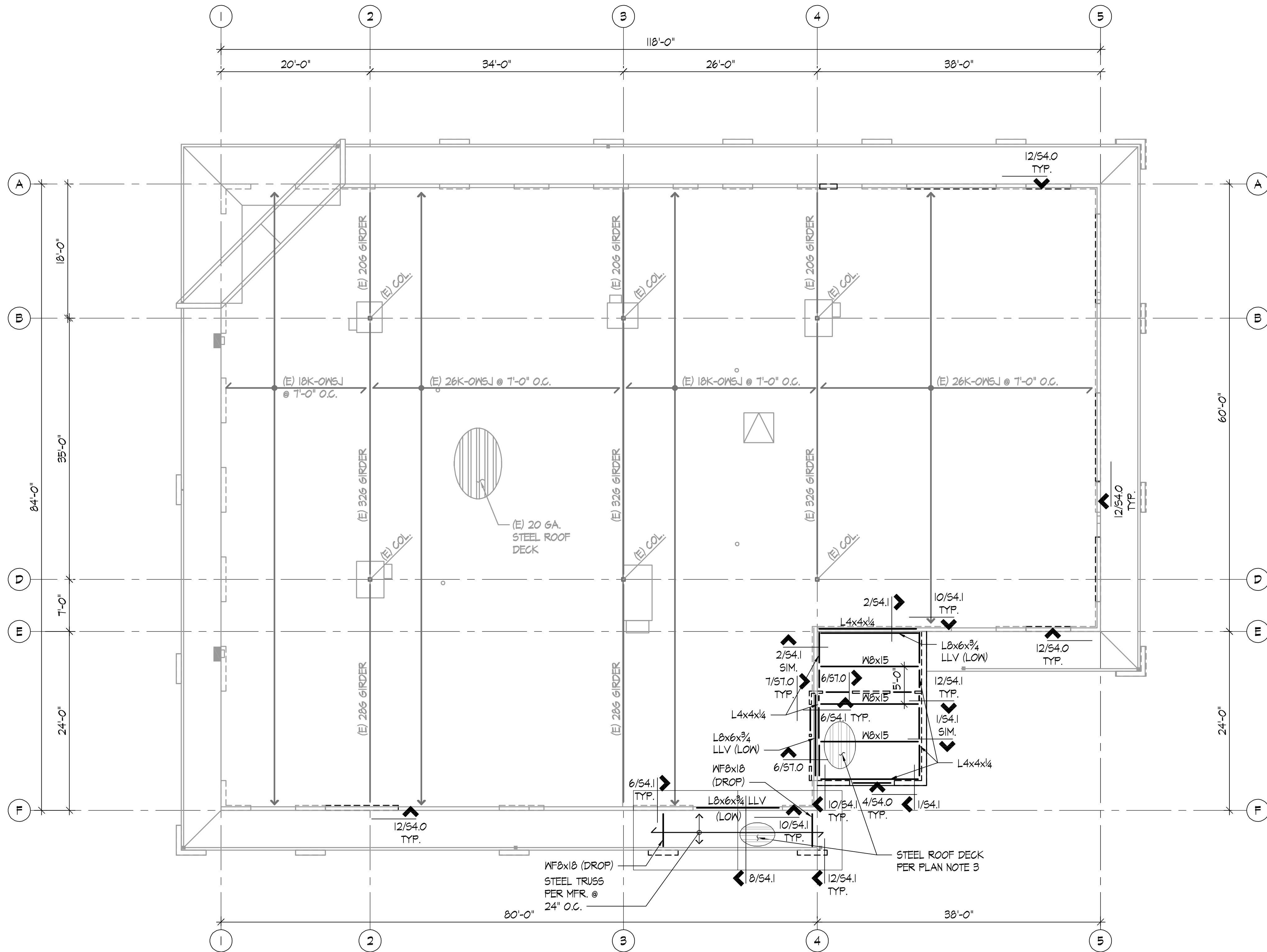
Drawing

## S2.0

Job Number: 20046.02



File: 046-1201.dwg    Plot Date: Wed, 07/22/2020 6:00 am



## ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

### ROOF FRAMING NOTES:

- ALL DIMENSIONS AND ELEVATIONS ON THE STRUCTURAL PLANS ARE FOR GENERAL INFORMATION ONLY AND SHALL BE VERIFIED BY THE CONTRACTOR WITH THE ARCHITECTURAL DRAWINGS BEFORE CONSTRUCTION BEGINS. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT AND ENGINEER IMMEDIATELY.
- SEE SHEETS S1.0 AND S1.1 FOR GENERAL STRUCTURAL NOTES AND ABBREVIATIONS. SEE SHEETS S4.0 & S4.1 FOR TYPICAL MASONRY DETAILS, & S7.0 FOR TYPICAL LIGHT GAUGE DETAILS.
- TYPICAL ROOF FRAMING CONSISTS OF 20 GA. 1/2" DEEP WIDE TYPE B ROOF DECK LAID OVER WF BEAM & ANGLES W/ 6'-0" MAX. SPAN, U.O.N.
- FASTEN ROOF STEEL PAN DECK TO WF BEAMS W/ RUDDLE WELDS @ 6" O.C. TO ALL SUPPORTS & PUNCHLOK/DELTA GRIP @ 12" O.C. AT SIDE SEAMS PER DETAIL 4/S4.1, U.O.N.
- PROVIDE SOLID OR BUILT-UP POSTS PER 6/S7.0, U.O.N. BENEATH THE ENDS OF ALL ROOF BEAMS FOR FULL BEARING LIGHT GAUGE STEEL HEADERS OR JOISTS.
- FOR TOP TRACK SPLICE SEE DETAIL 11/S7.0.

### LEGEND:

- INDICATES FRAMING DIRECTION  
--- INDICATES EXTENT OF FRAMING  
--- INDICATES CMU BEARING WALL/PIER

Date:	For:
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2017  
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Tenant Improvements for:  
**Stillaguamish Tribal Courthouse**  
17014 59th Avenue NE  
Arlington, Washington  
Contract:  
**ROOF FRAMING PLAN**

Drawing:
<b>S2.1</b>
Job Number:
<b>20046.02</b>

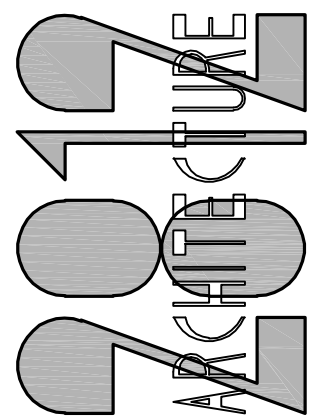
			<p><b>NOTE:</b> LIGHT GAUGE STUD WALL AND ATTACHMENT PER 3/5T.O AT SIM. CONDITION</p>
TYPICAL PERIMETER MASONRY WALL FOUNDATION <div>SCALE: NONE</div> <div>1</div>	TYPICAL INTERIOR MASONRY WALL FOUNDATION <div>SCALE: 1"=1'-0"</div> <div>2</div>	DETAIL <div>SCALE: NONE</div> <div>3</div>	DETAIL <div>SCALE: 1"=1'-0"</div> <div>4</div>
<p><b>NOTE:</b> SEE DETAIL 1/53.0 FOR INFORMATION NOT NOTED</p>			
TYPICAL MASONRY PIER FOOTING <div>SCALE: NONE</div> <div>5</div>	DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>6</div>	DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>7</div>	DETAIL <div>SCALE: NONE</div> <div>8</div>
DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>9</div>	DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>10</div>	DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>11</div>	DETAIL <div>SCALE: 3/4"=1'-0"</div> <div>12</div>

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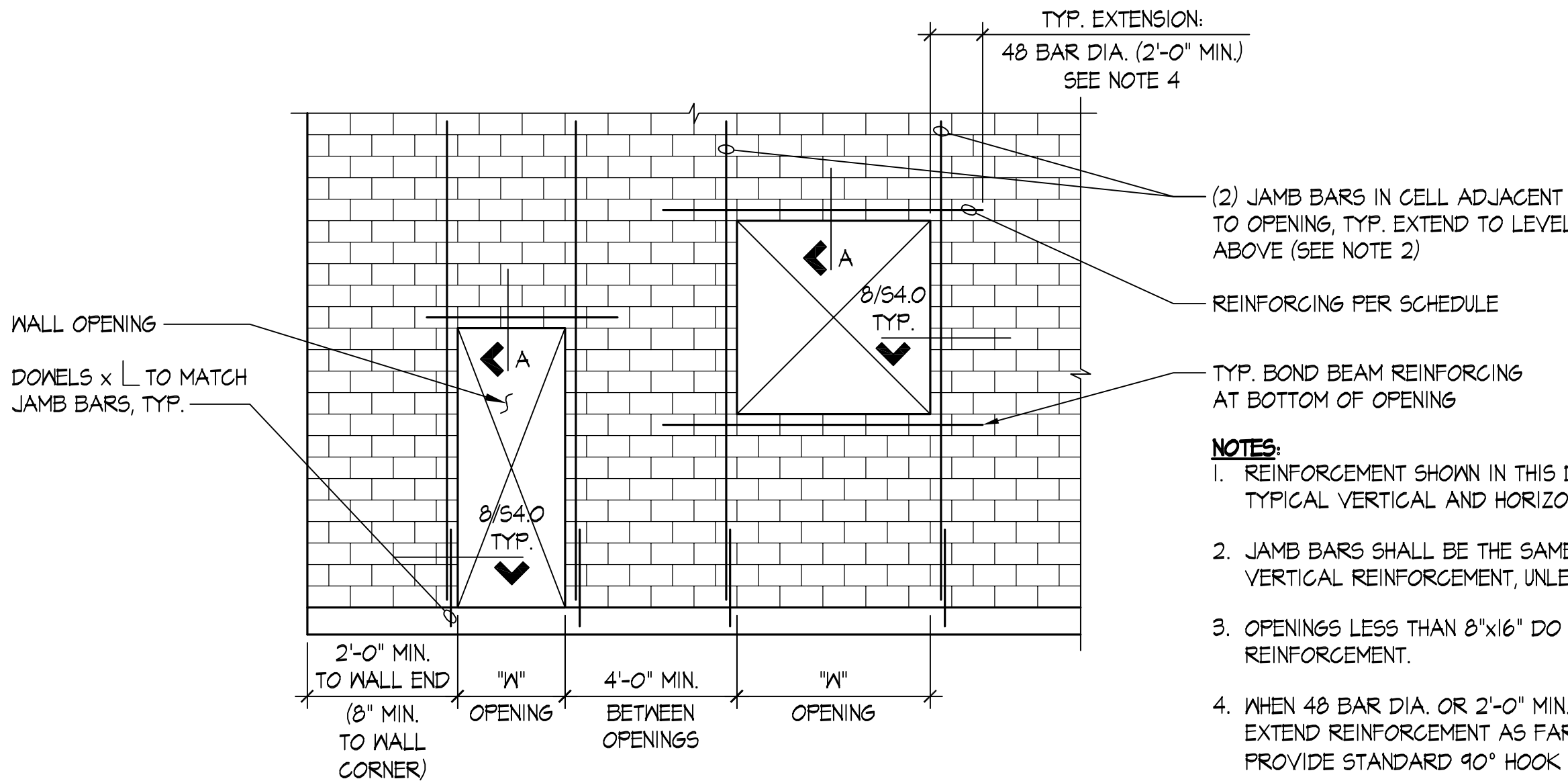
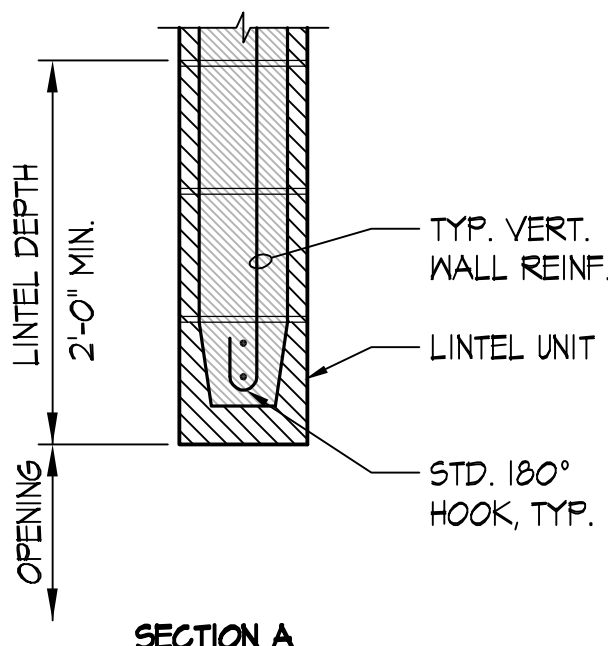
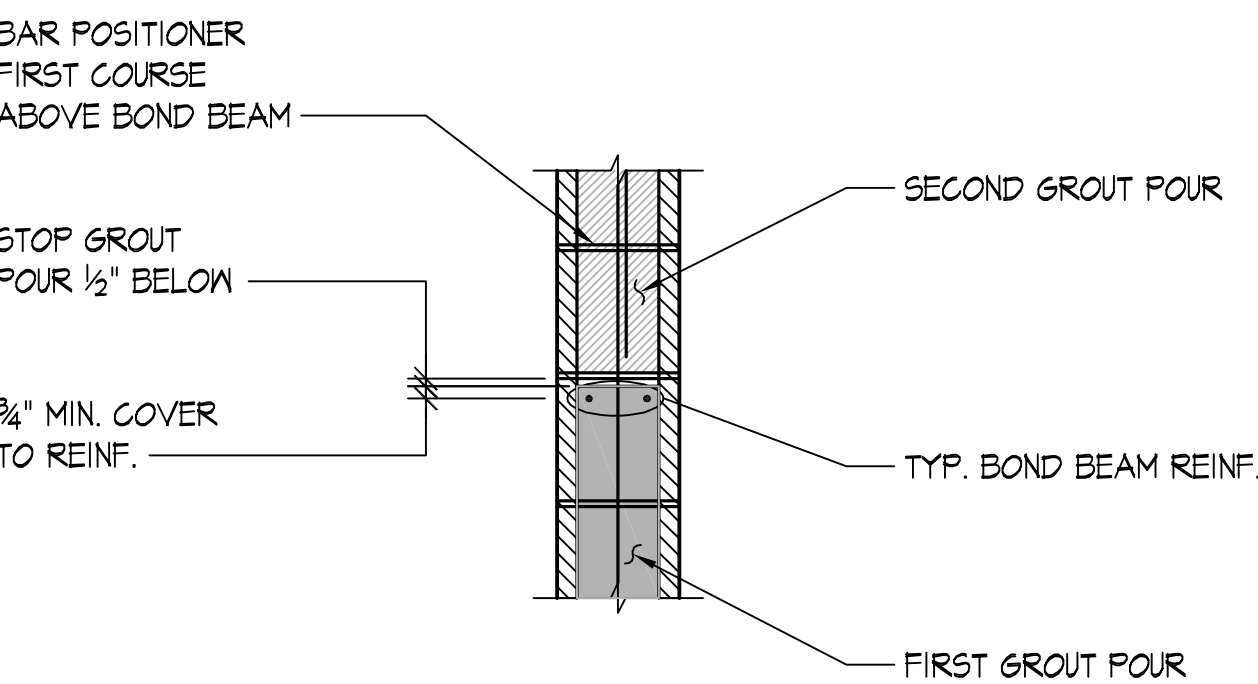
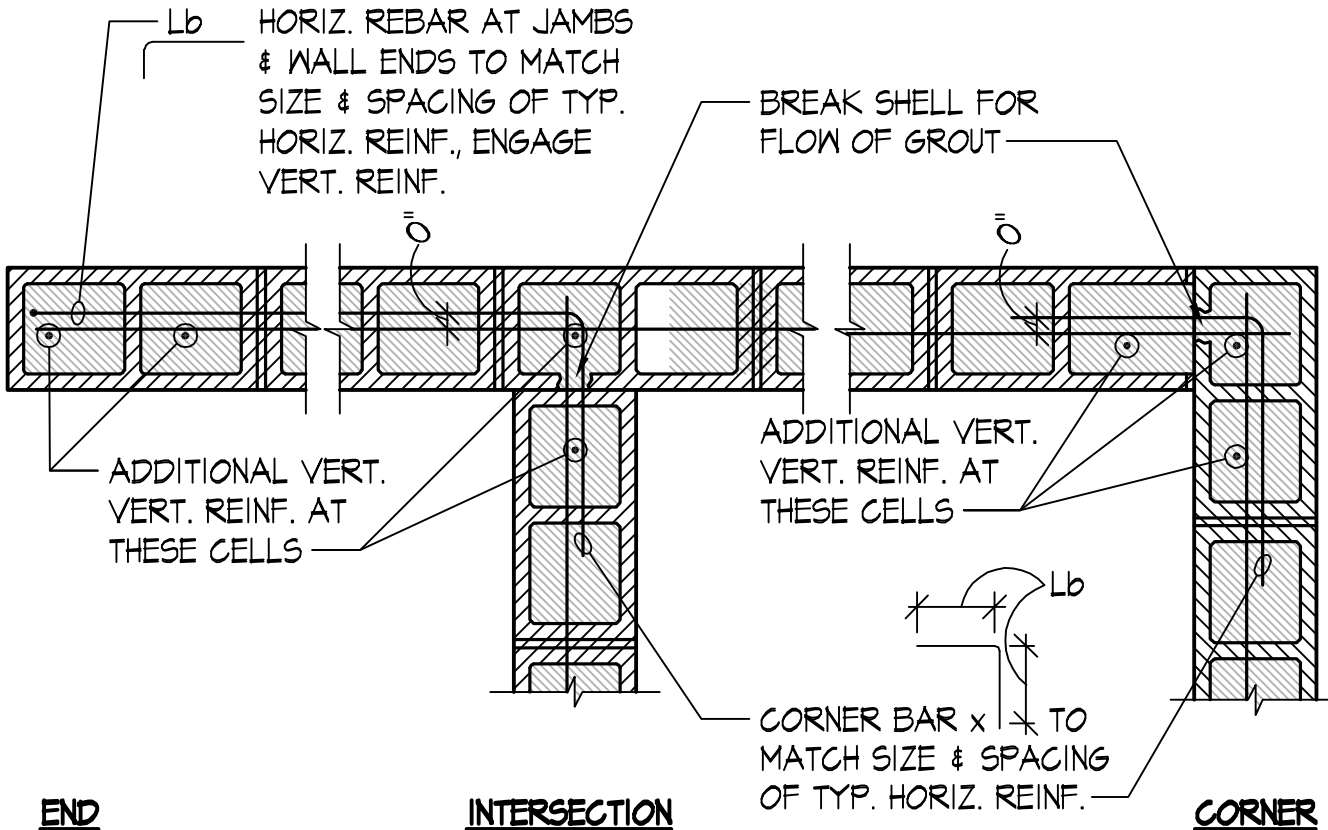
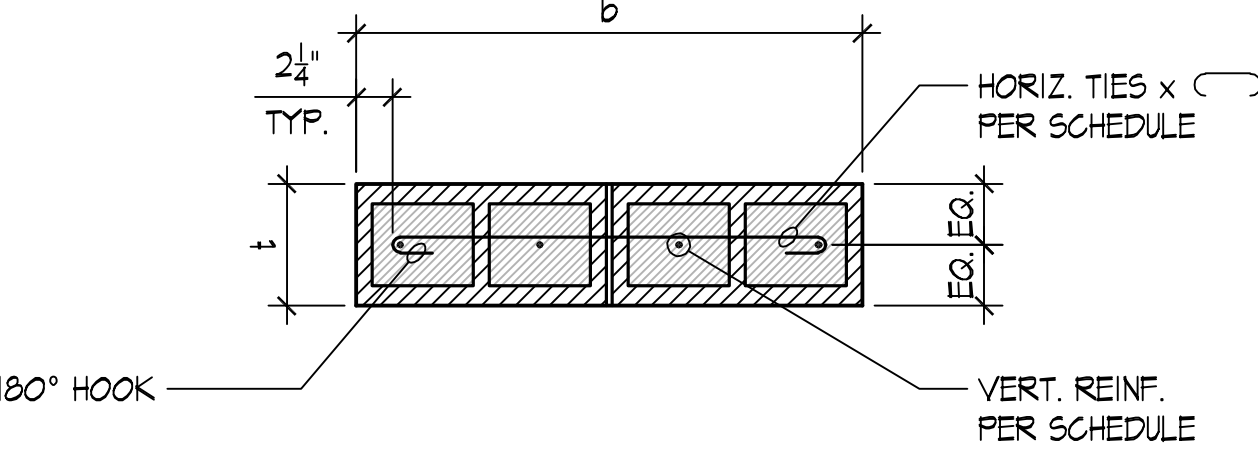
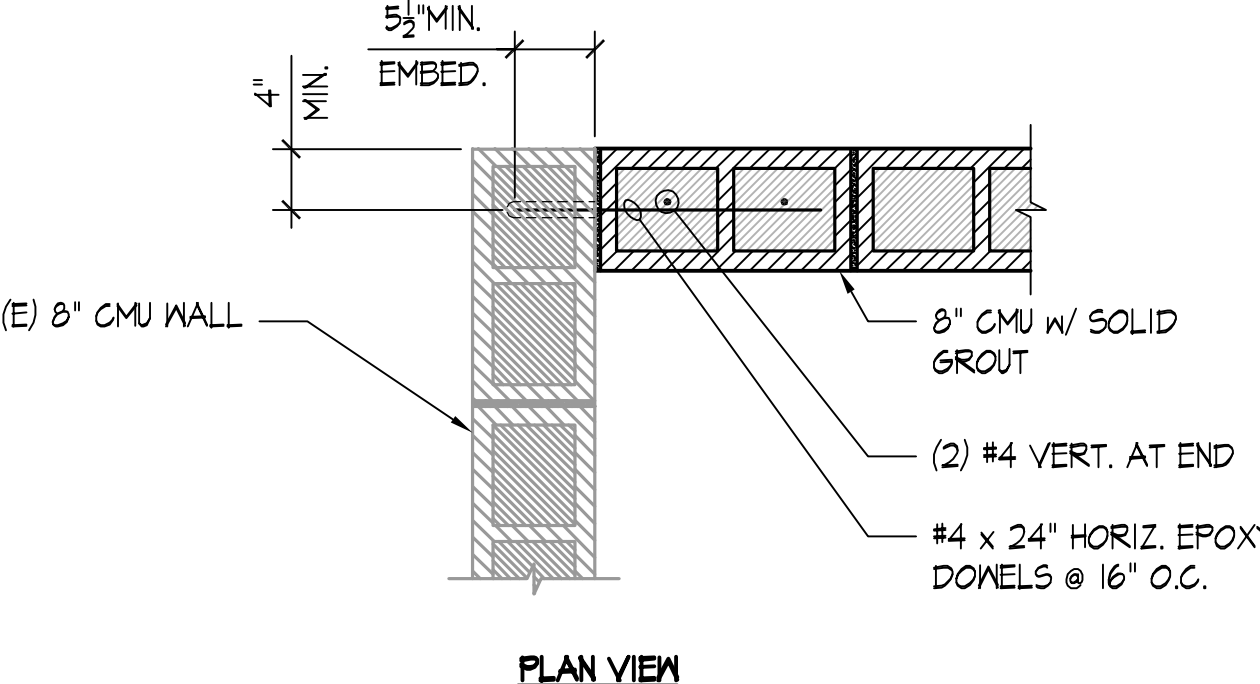
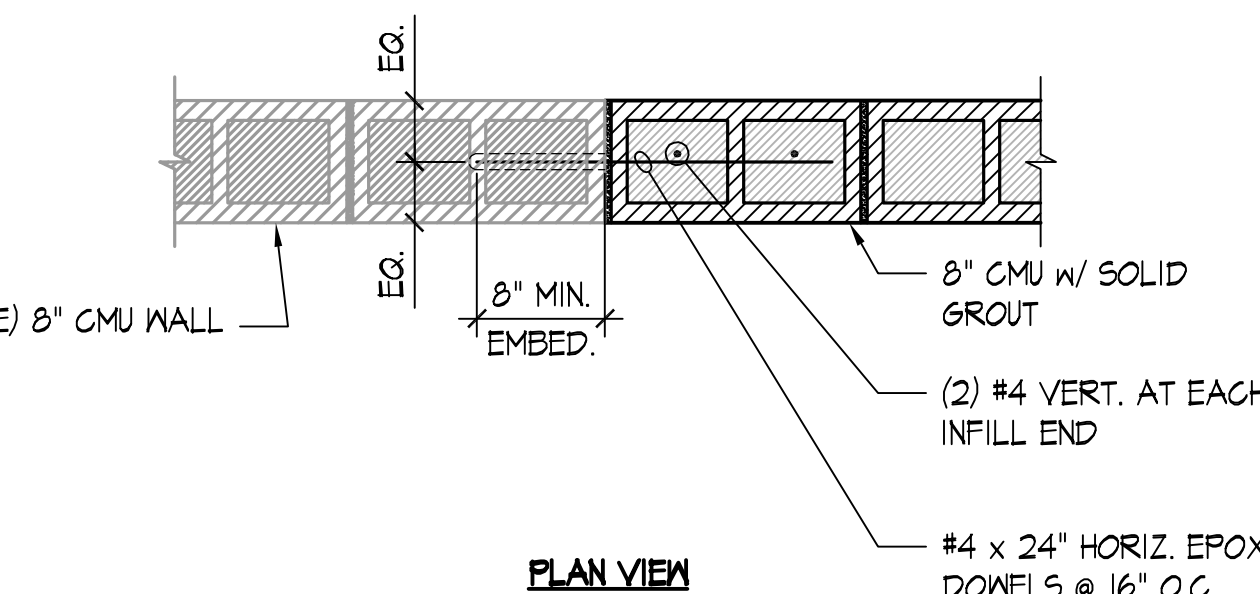
CONTRACTOR:  
 DETAILS

Drawing:  
**S3.0**

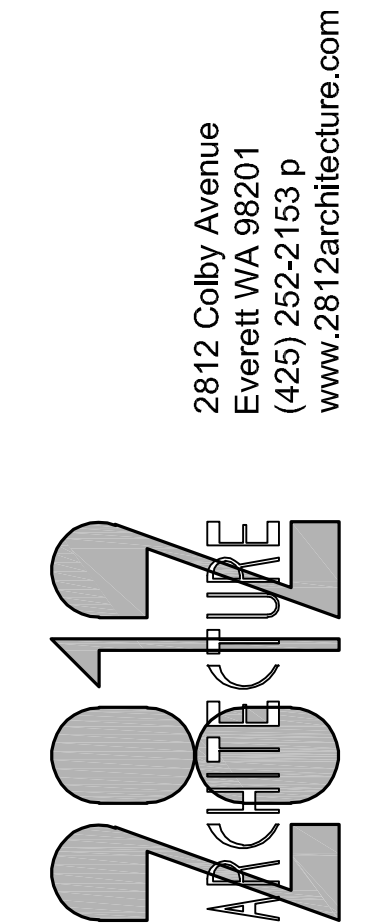
Job Number:  
 20046.02



File: 046-S400.dwg Plotter: West, 07/22/2020 6:55 am

						<table><caption>LINTEL REINFORCING SCHEDULE</caption><tr><th>OPENING "W"</th><th>LINTEL REINF.</th></tr><tr><td>LESS THAN 6'-0"</td><td>(2) #5</td></tr><tr><td>6'-0" TO LESS THAN 10'-0"</td><td>(2) #6</td></tr></table>		OPENING "W"	LINTEL REINF.	LESS THAN 6'-0"	(2) #5	6'-0" TO LESS THAN 10'-0"	(2) #6						
OPENING "W"	LINTEL REINF.																		
LESS THAN 6'-0"	(2) #5																		
6'-0" TO LESS THAN 10'-0"	(2) #6																		
DETAIL	SCALE: NONE	TYPICAL REINFORCEMENT AROUND OPENINGS IN MASONRY WALLS		SCALE: NONE		4													
<p><b>MASONRY REINFORCING SPLICE AND DEVELOPMENT LENGTH SCHEDULE</b></p> <p>DEVELOPMENT &amp; SPLICING OF REINFORCING IN GROUTED MASONRY CONST. FOR GR. 40 &amp; GR. 60 REBAR</p> <table><tr><th colspan="2">TENSION LAP SPLICE ("Lb")</th></tr><tr><th>BAR SIZE</th><th>48db</th></tr><tr><td># 3</td><td>18"</td></tr><tr><td># 4</td><td>24"</td></tr><tr><td># 5</td><td>30"</td></tr><tr><td># 6</td><td>36"</td></tr></table>		TENSION LAP SPLICE ("Lb")		BAR SIZE	48db	# 3	18"	# 4	24"	# 5	30"	# 6	36"					<p><b>NOTES:</b></p> <ol style="list-style-type: none"><li>PLACE VERTICAL REINFORCING AT CENTER OF WALL U.O.N.</li><li>USE DOUBLE OPEN END BOND BEAM UNITS, SINGLE OPEN END UNITS AT CORNERS &amp; ENDS OF WALL.</li><li>WHERE BOND BEAM IN ADJACENT WALLS OCCUR AT DIFFERENT LEVELS, RETURN ONE OF THE BEAMS 2'-0" AROUND CORNER</li></ol>	
TENSION LAP SPLICE ("Lb")																			
BAR SIZE	48db																		
# 3	18"																		
# 4	24"																		
# 5	30"																		
# 6	36"																		
MASONRY REINFORCING SPLICE LENGTH SCHEDULE		SCALE: NONE	5	TYPICAL HORIZONTAL CONSTRUCTION JOINT IN MASONRY WALL		SCALE: NONE	6	DETAIL		SCALE: NONE	7	TYPICAL MASONRY WALL ENDS, INTERSECTIONS & CORNERS		SCALE: NONE	8				
		 <table><caption>PIER SCHEDULE</caption><tr><th>MARK</th><th>t</th><th>b</th><th>VERTICAL REINFORCING</th><th>HORIZONTAL TIES</th></tr><tr><td>8PIER48</td><td>8"</td><td>48"</td><td>(6) #6</td><td>#4 @ 8" O.C.</td></tr></table> <p><b>NOTE:</b> CONTINUOUS VERTICAL REINFORCING (w/ LAP SPLICES PER SCHEDULE) FROM FOOTING TO TOP OF PIER</p>		MARK	t	b	VERTICAL REINFORCING	HORIZONTAL TIES	8PIER48	8"	48"	(6) #6	#4 @ 8" O.C.			<p><b>NOTE:</b> LOCATE HORIZ. EPOXY DOWELS WITHIN 8" FROM TOP OF INFILL WALL OPNG.</p> 			
MARK	t	b	VERTICAL REINFORCING	HORIZONTAL TIES															
8PIER48	8"	48"	(6) #6	#4 @ 8" O.C.															
DETAIL		SCALE: NONE	9	TYPICAL MASONRY PIER		SCALE: NONE	10	DETAIL		SCALE: NONE	11	TYPICAL MASONRY WALL INFILL		SCALE: NONE	12				

For:	PERMIT SET	
Date:	4/22/20	7/22/20
	BID SET	



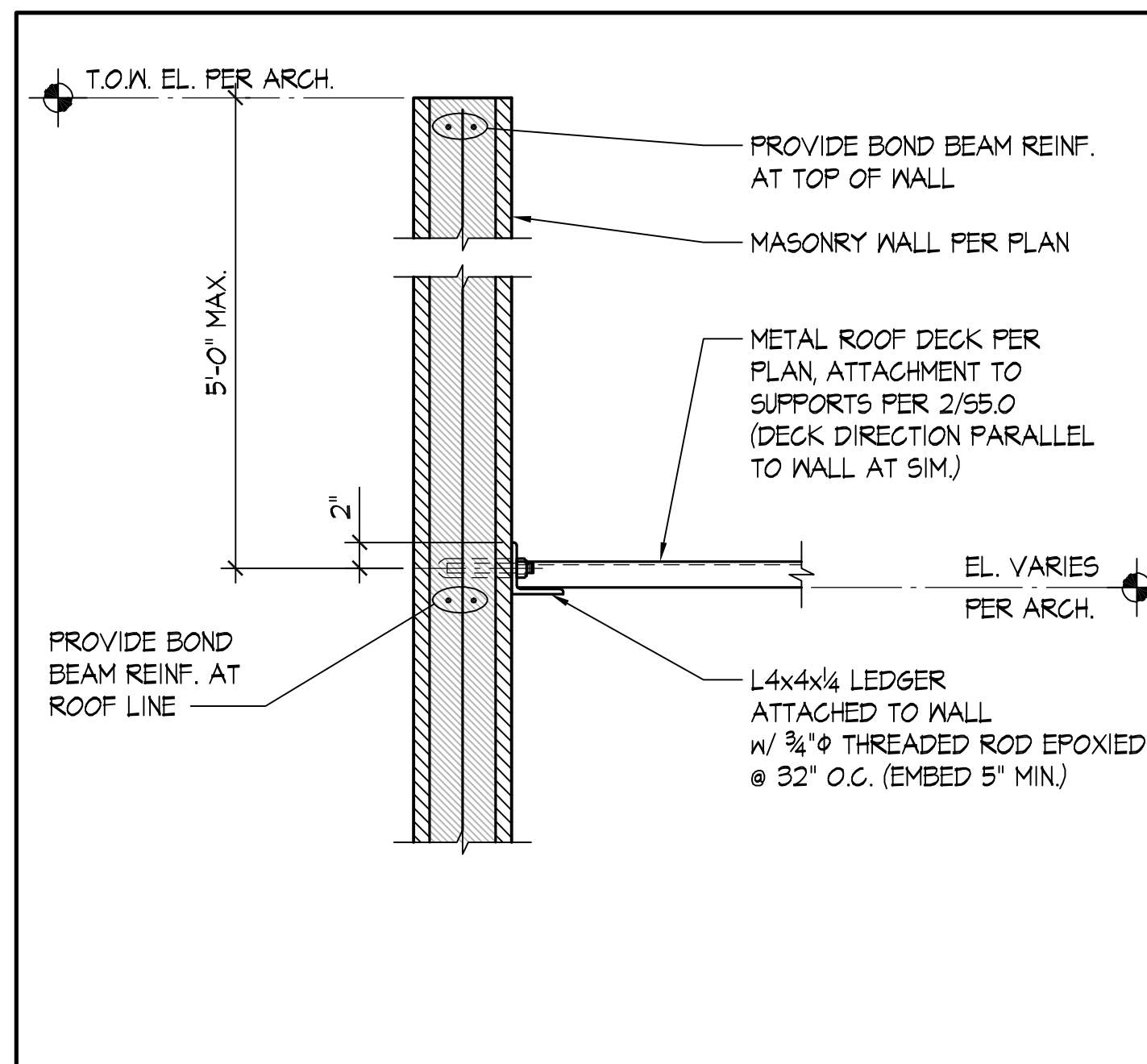
Tenant Improvements for:  
**Stiliaguamish Tribal Courthouse**  
17014 58th Avenue NE  
Arlington, Washington

Contractor: TYPICAL MASONRY DETAILS

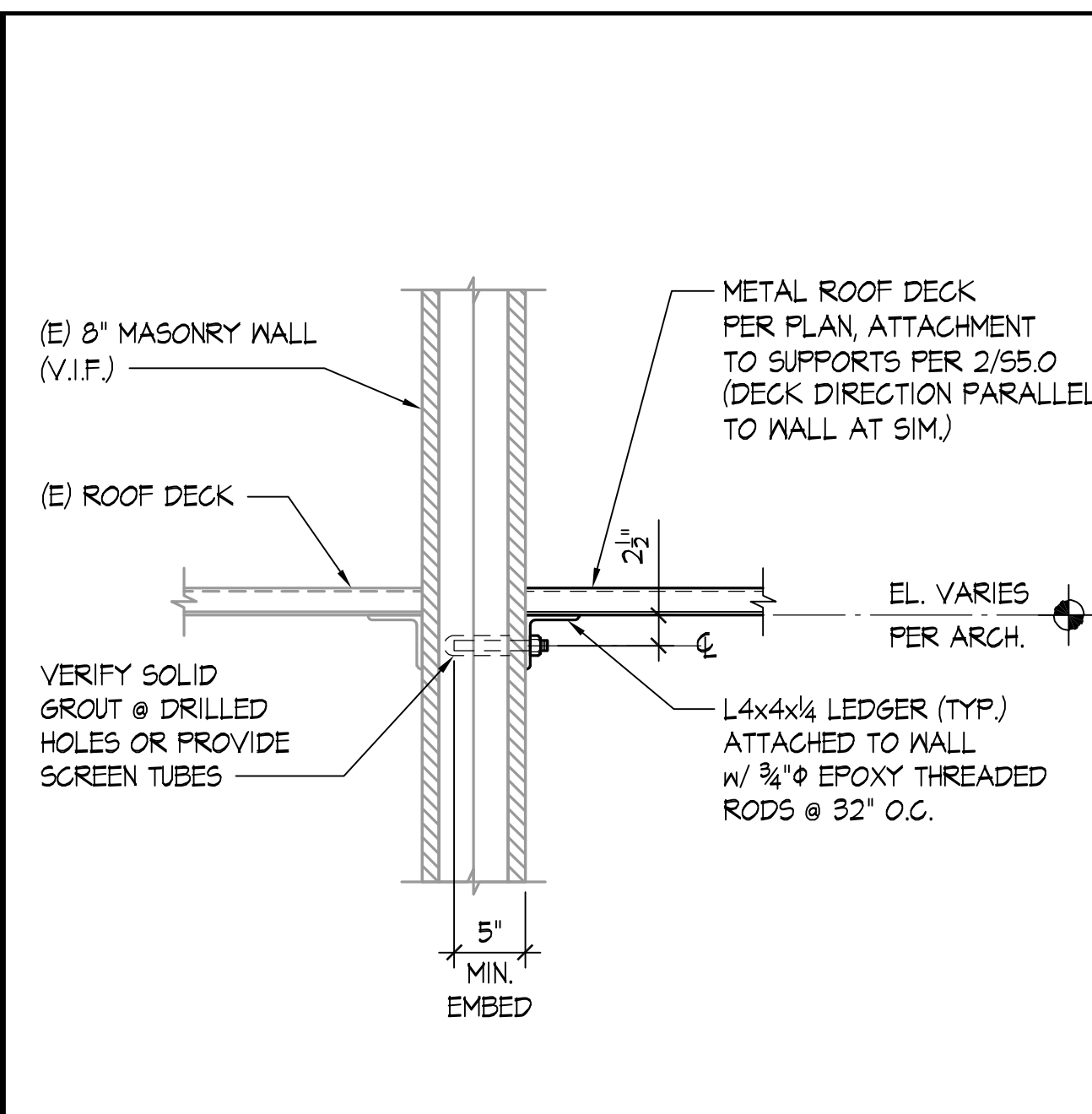
Drawing: **S4.0**

Job Number: 20046.02

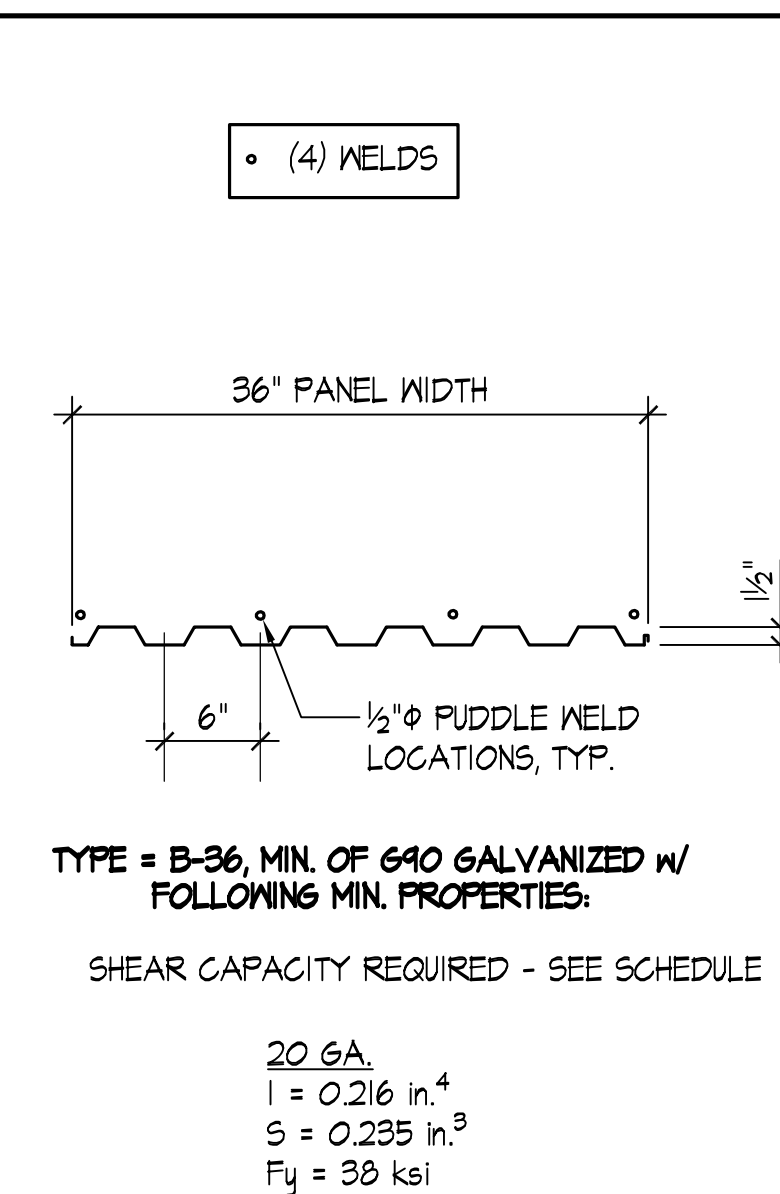




TYPICAL PERIMETER MASONRY WALL AT ROOF DECK



TYPICAL INTERIOR MASONRY WALL AT ROOF DECK



TYPICAL 1½" ROOF DECK

ROOF DECK DIAPHRAGM CONNECTION SCHEDULE			
PUDDLE WELDS PER PANEL - ENDS	PUDDLE WELDS PER PANEL - INTERMEDIATE	PUNCHLOK/DELTA GRIP SIDE SEAMS	CAPACITY PL
4	4	12" O.C.	650

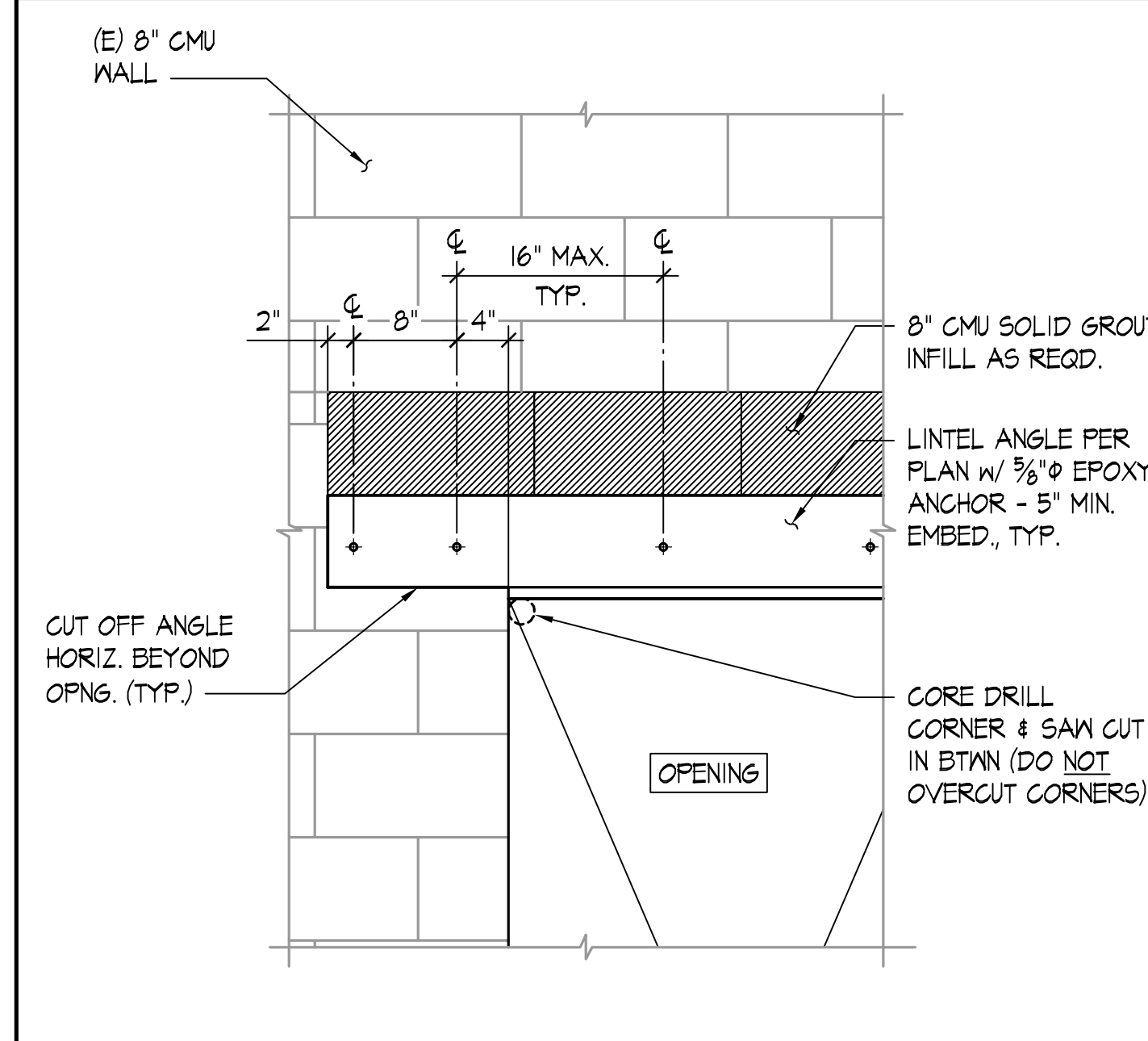
- NOTES:**
1. MAXIMUM DECK SPAN = 6'-0" CLEARSAN (TWO OR MORE CONTINUOUS SPANS).
  2. PROVIDE 1/2"x6" PUDDLE WELDS PER PANEL TO ALL SUPPORTS PERPENDICULAR TO DECK FLUTES (PER SCHEDULE), U.O.N.
  3. PROVIDE 1/2"x6" PUDDLE WELDS @ 6" O.C. TO ALL LEDGERS PARALLEL TO DECK FLUTES.
  4. DECK TYPE MUST MEET OR EXCEED CRITERIA LISTED, INCLUDING I.G.C. OR IAPMO RESEARCH REPORT ALLOWABLE SHEAR LOADS.

TYPE = B-36, MIN. OF 690 GALVANIZED W/  
FOLLOWING MIN. PROPERTIES:

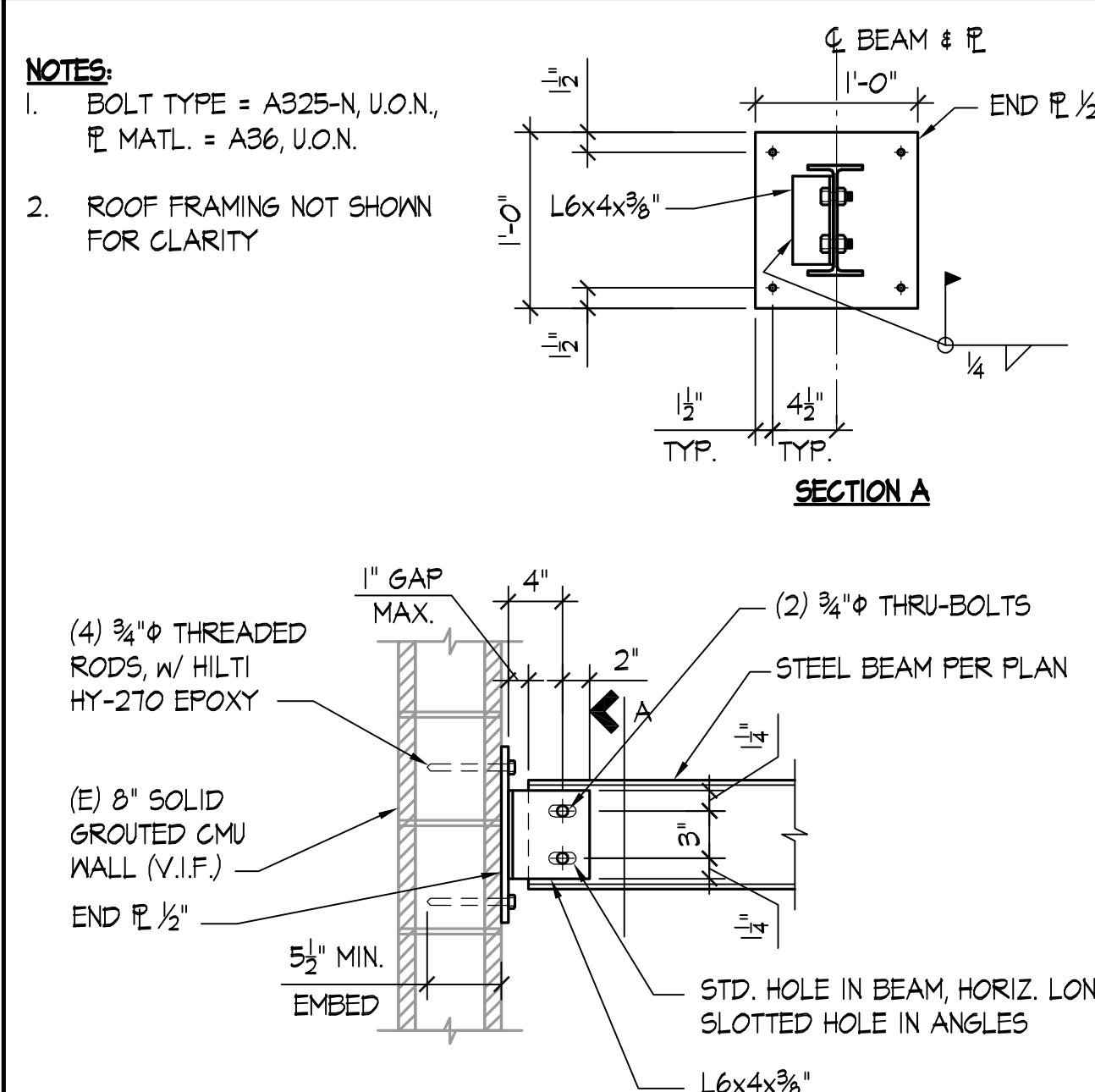
SHEAR CAPACITY REQUIRED - SEE SCHEDULE

20 GA.  
 $I = 0.216 \text{ in.}^4$   
 $S = 0.235 \text{ in.}^3$   
 $F_y = 38 \text{ ksi}$

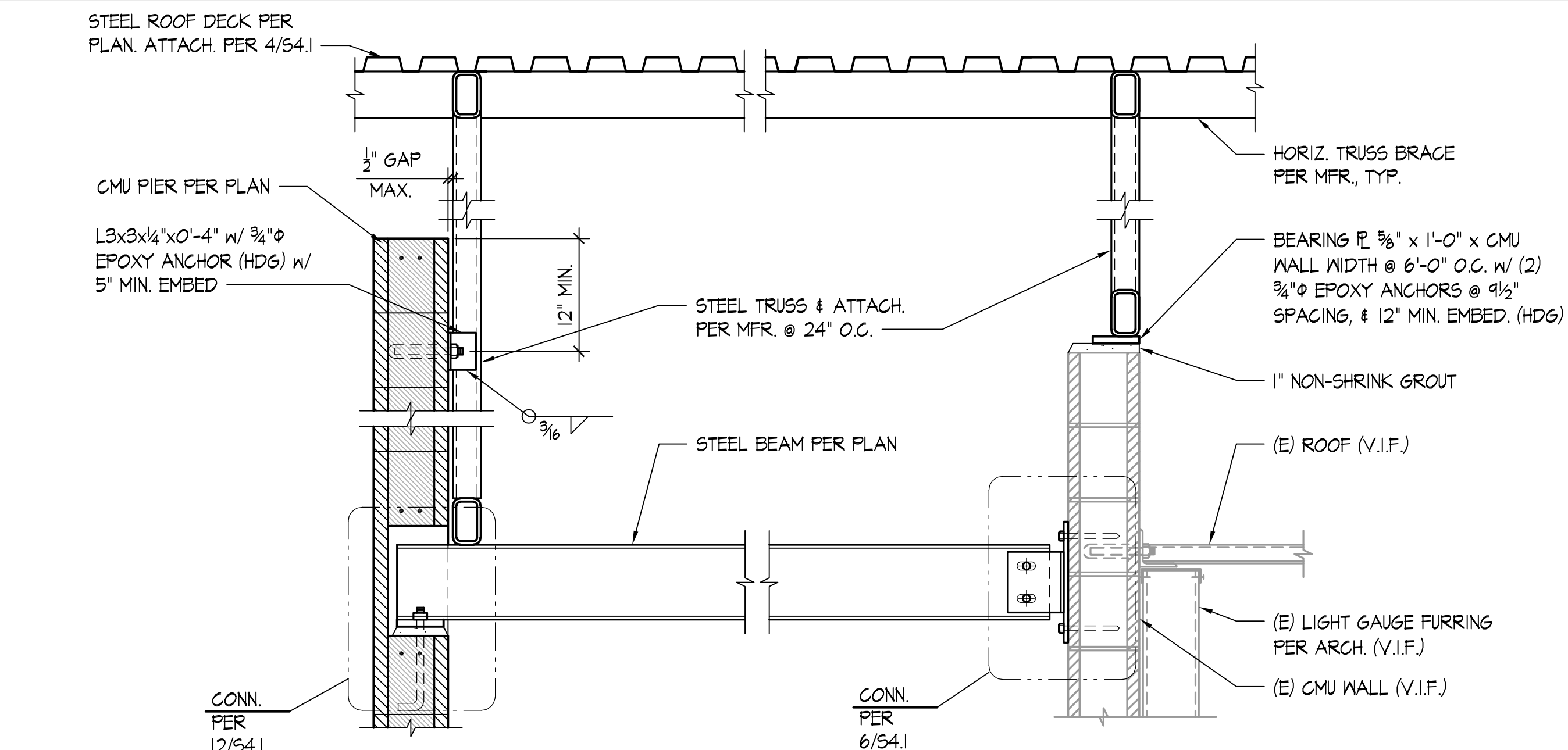
SCALE: NONE



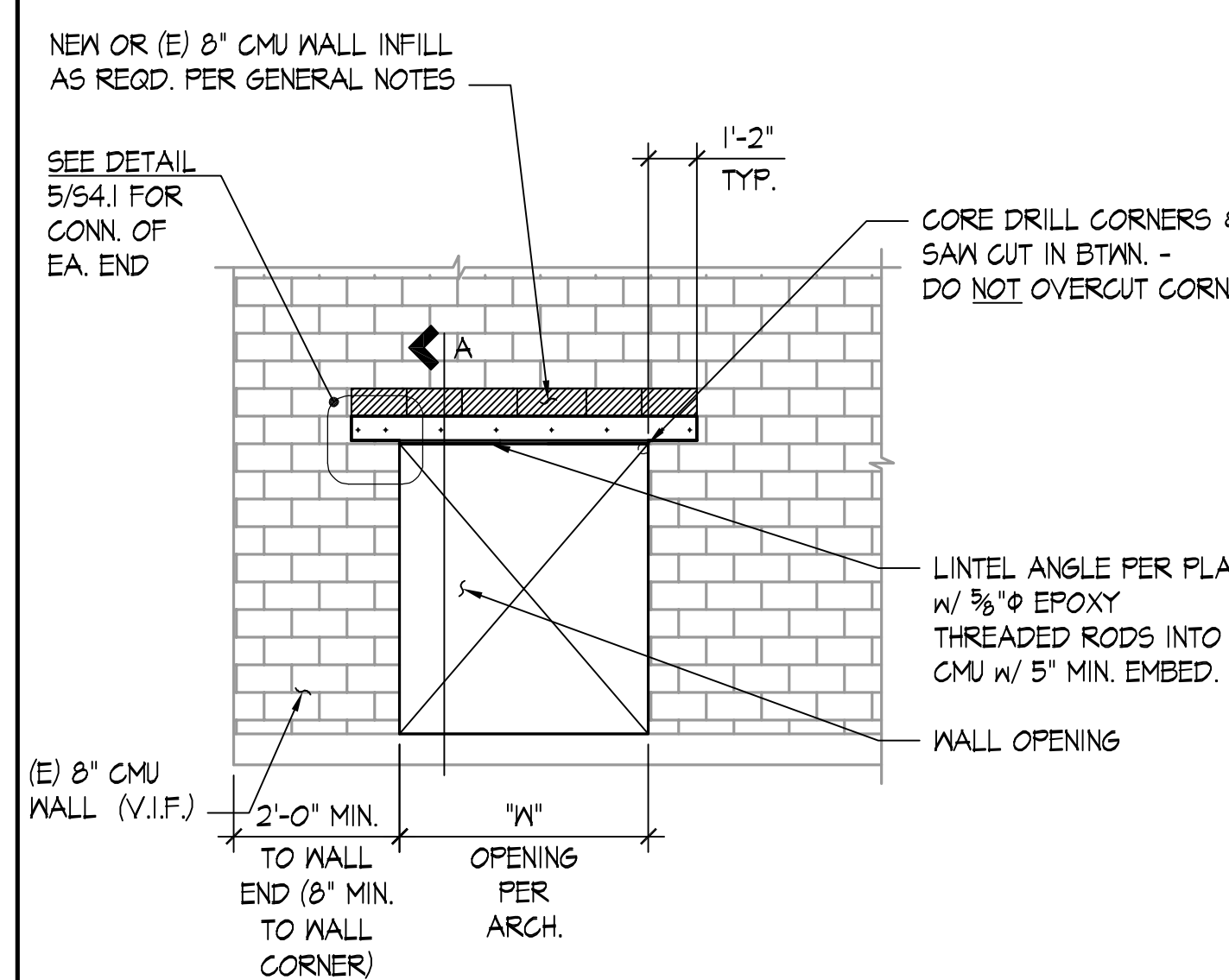
### LINTEL END DETAIL



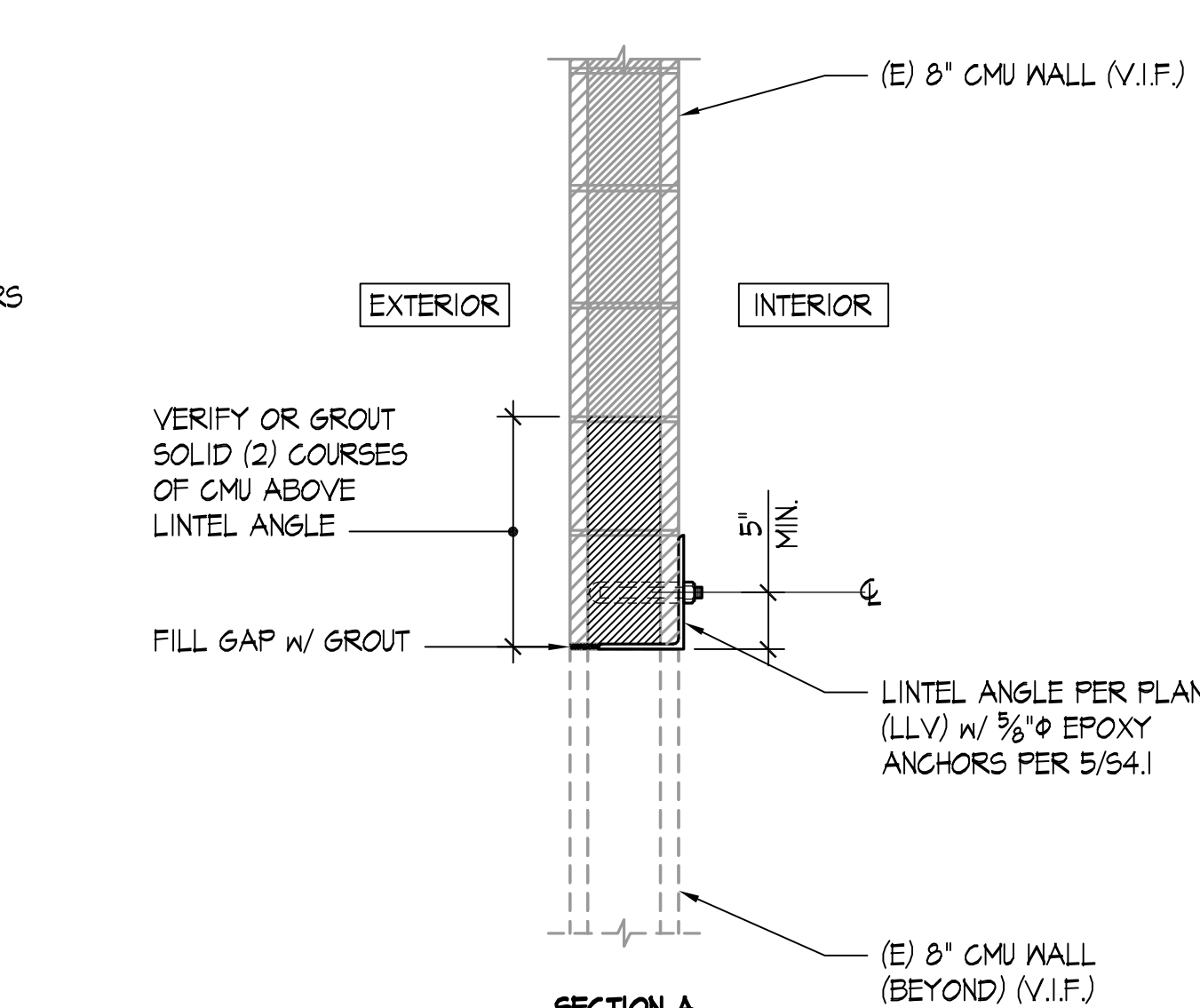
## DETAIL



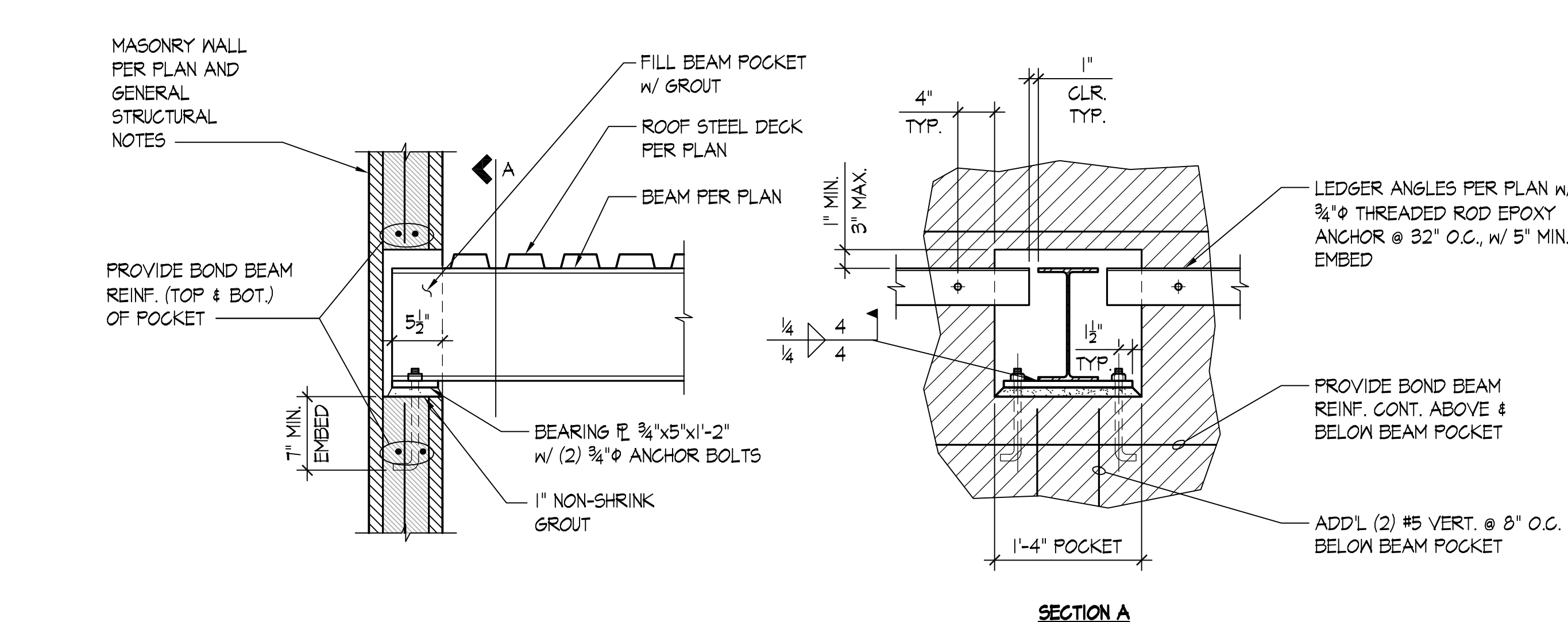
## DETAIL



### LINTEL STEEL ANGLE DETAIL



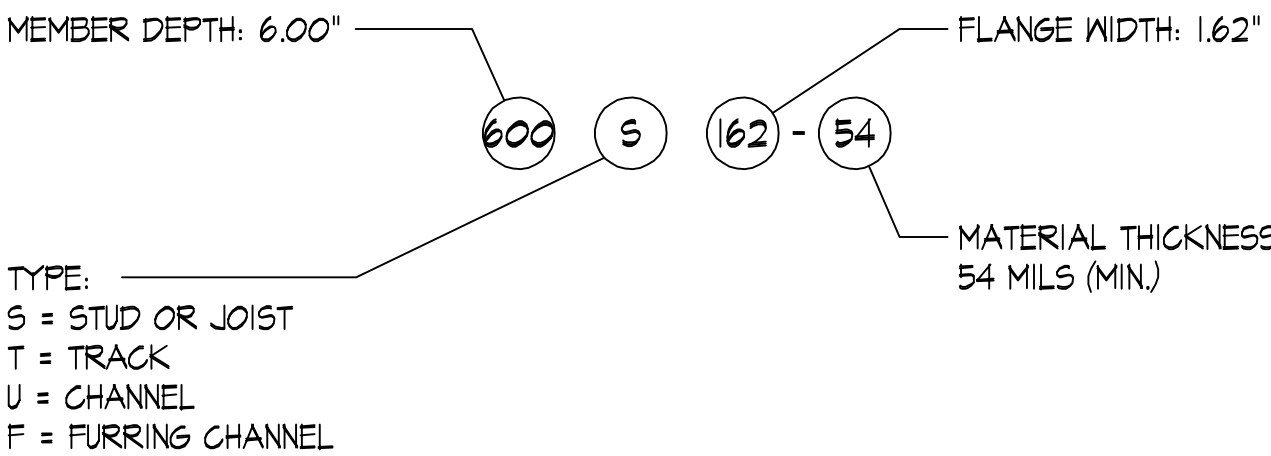
SCALE: NONE



TYPICAL STEEL BEAM POCKET IN MASONRY WALL

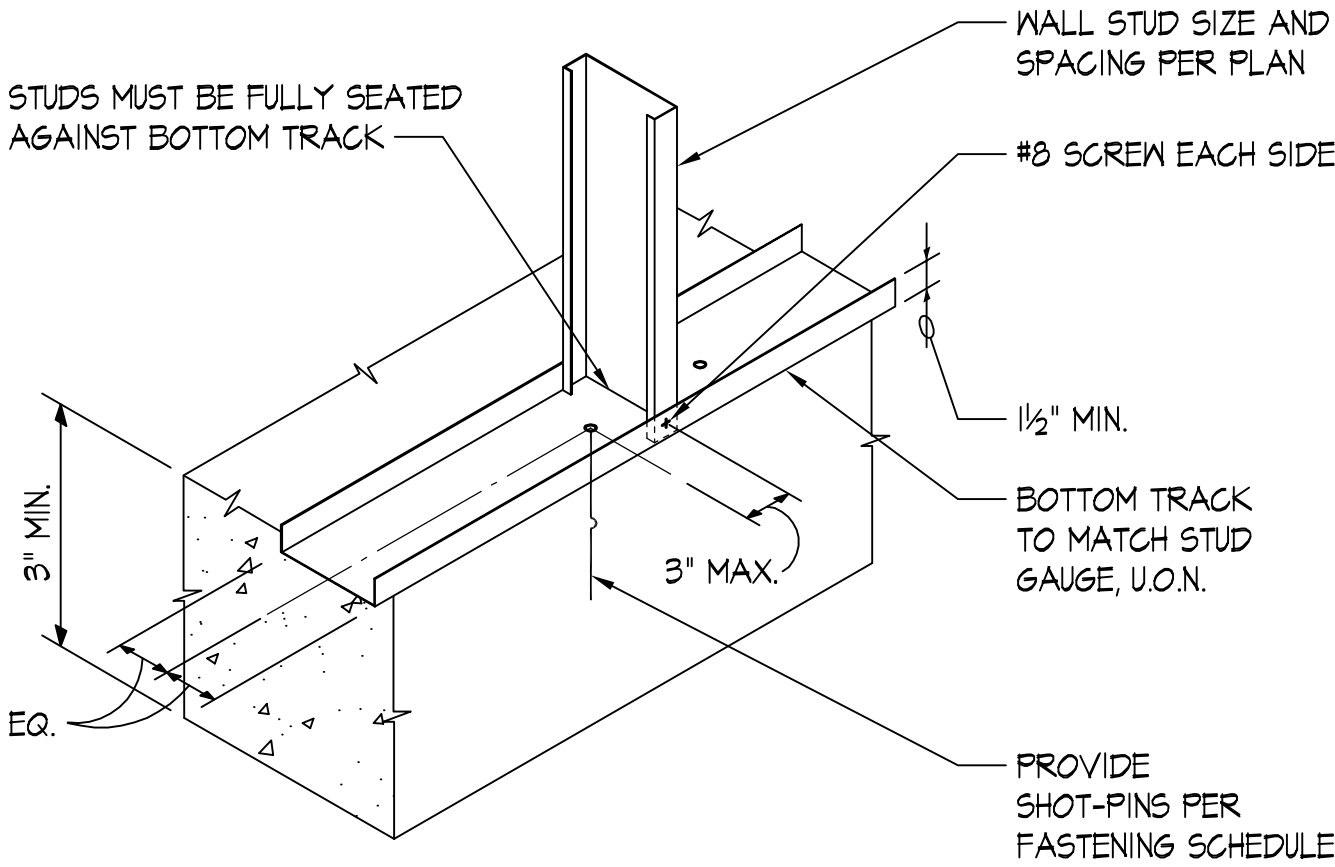
CONVERSION CHART		
MIL.	GAUGE	NOTES
30	20	DRYWALL
33	20	STRUCTURAL
43	18	
54	16	
68	14	
91	12	

EXAMPLE:



NOTE:

ALL "SHOT-PINS" SHALL BE POWDER ACTUATED FASTENERS PER THE STRUCTURAL GENERAL NOTES.



FASTENING SCHEDULE

WALL CONDITION	FASTENING REQUIREMENT
EXTERIOR STUD WALLS	(2) SHOT-PINS AT EACH STUD, ALSO PROVIDE (1) SHOT-PIN WITHIN 6" OF WALL END OR JAMB
INTERIOR STUD WALLS	(1) SHOT-PIN AT EACH STUD, ALSO PROVIDE (1) SHOT-PIN WITHIN 9" OF WALL END OR JAMB

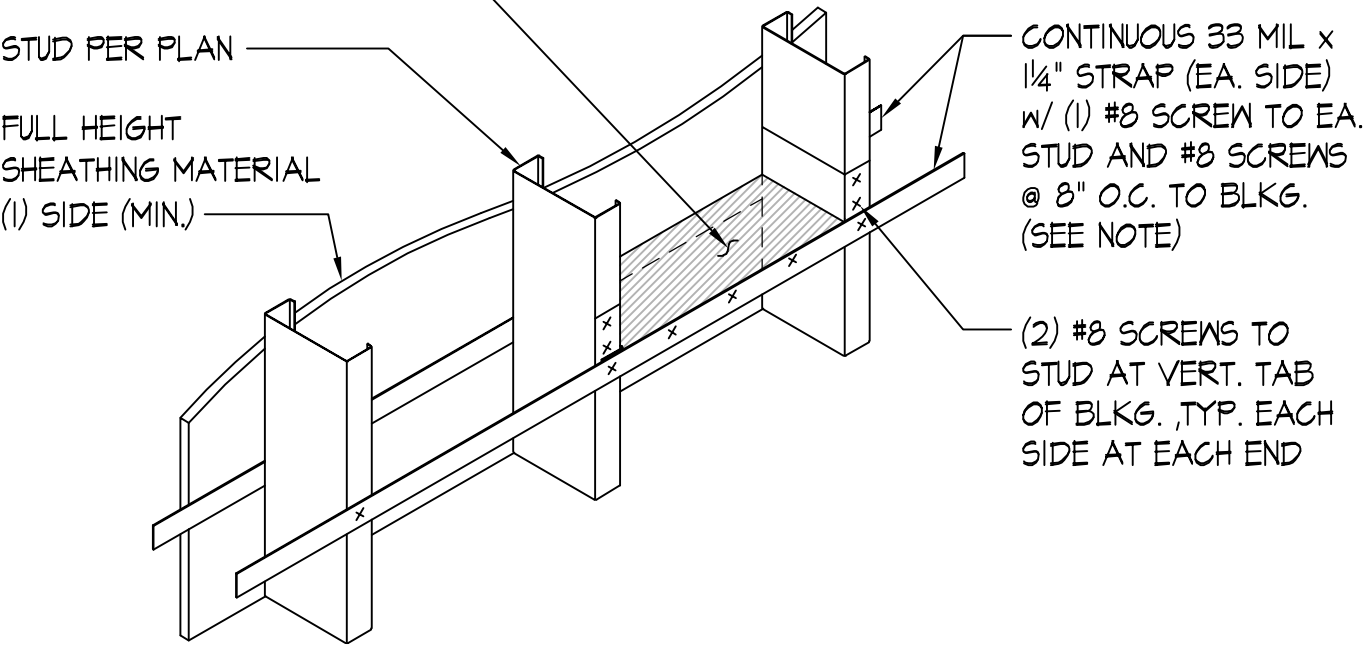
NOTE:

MAXIMUM STUD HEIGHT OF 20 FEET

NOTES:

- PROVIDE HORIZ. STRAPPING/BLKG. AT 3RD POINTS OF STUD HEIGHT
- STRAPPING/BLKG. MAY BE OMITTED WHERE SHTG. IS ATTACHED TO BOTH SIDES.

33 MIL. TRACK BLKG. @ 48" O.C. HORIZONTALLY, MAX. (SEE NOTE)



TYPICAL LIGHT GAUGE STEEL NOTATION (SSMA STANDARD)

SCALE: NONE

TYPICAL BOTTOM TRACK ATTACHMENT TO CONCRETE

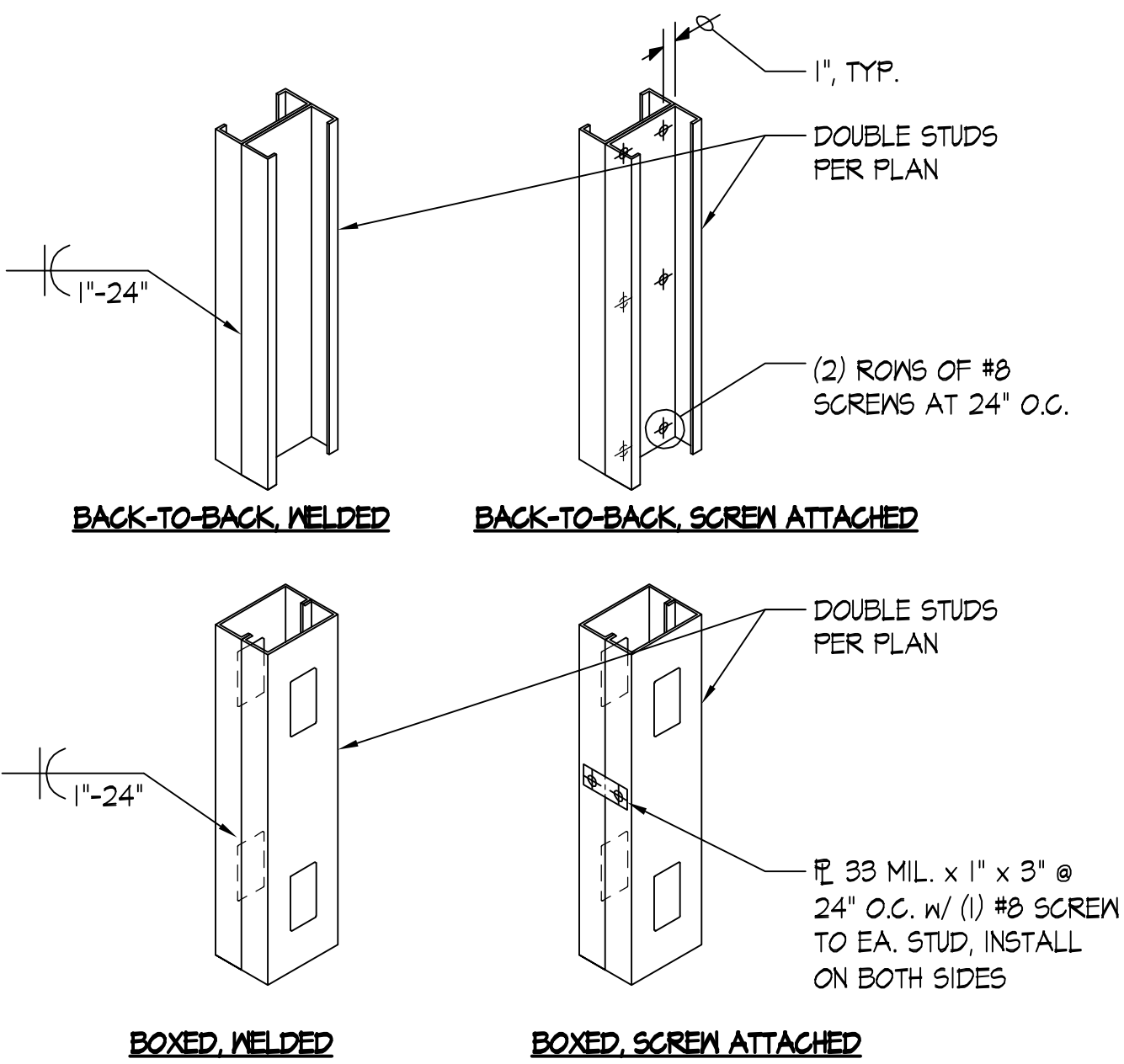
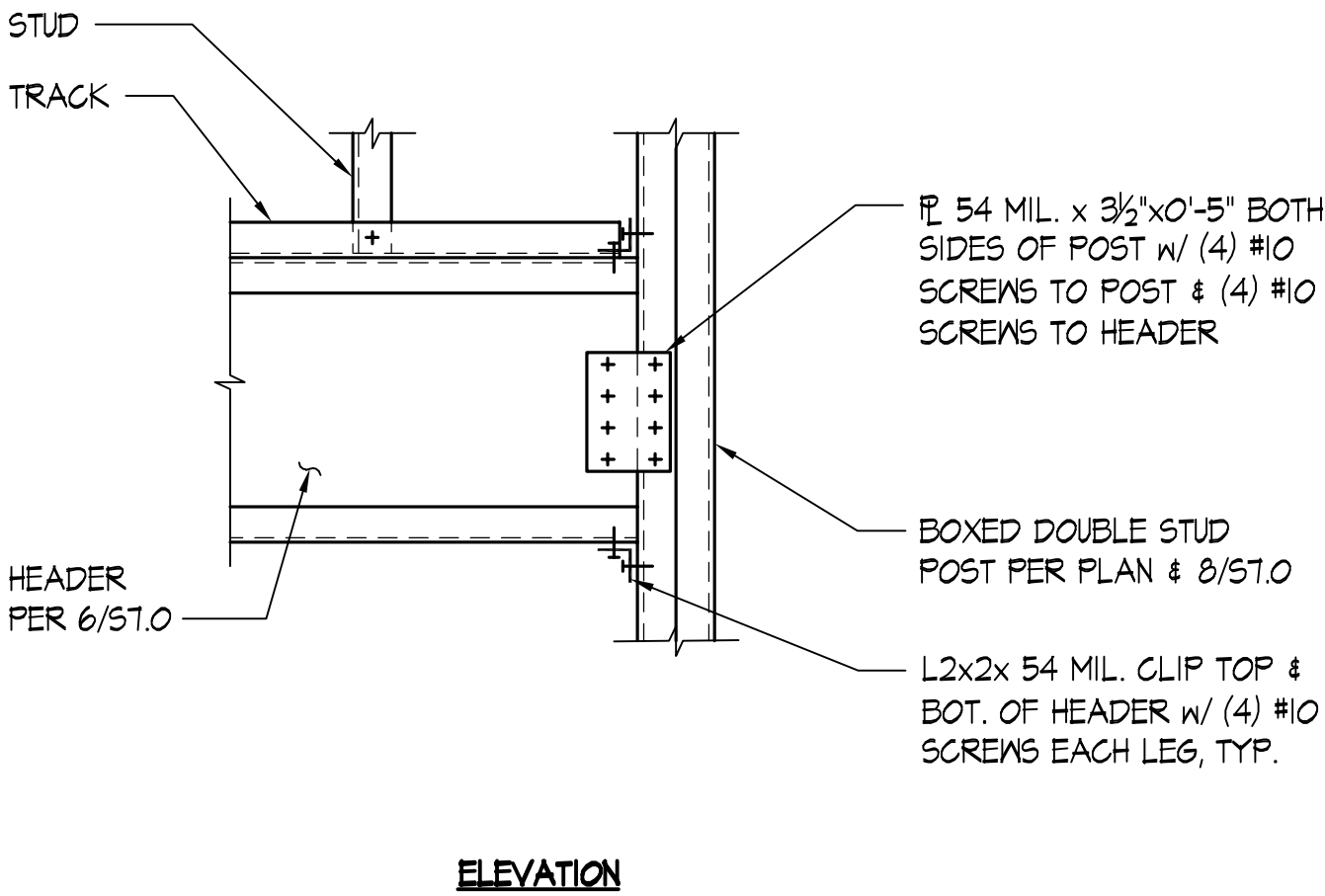
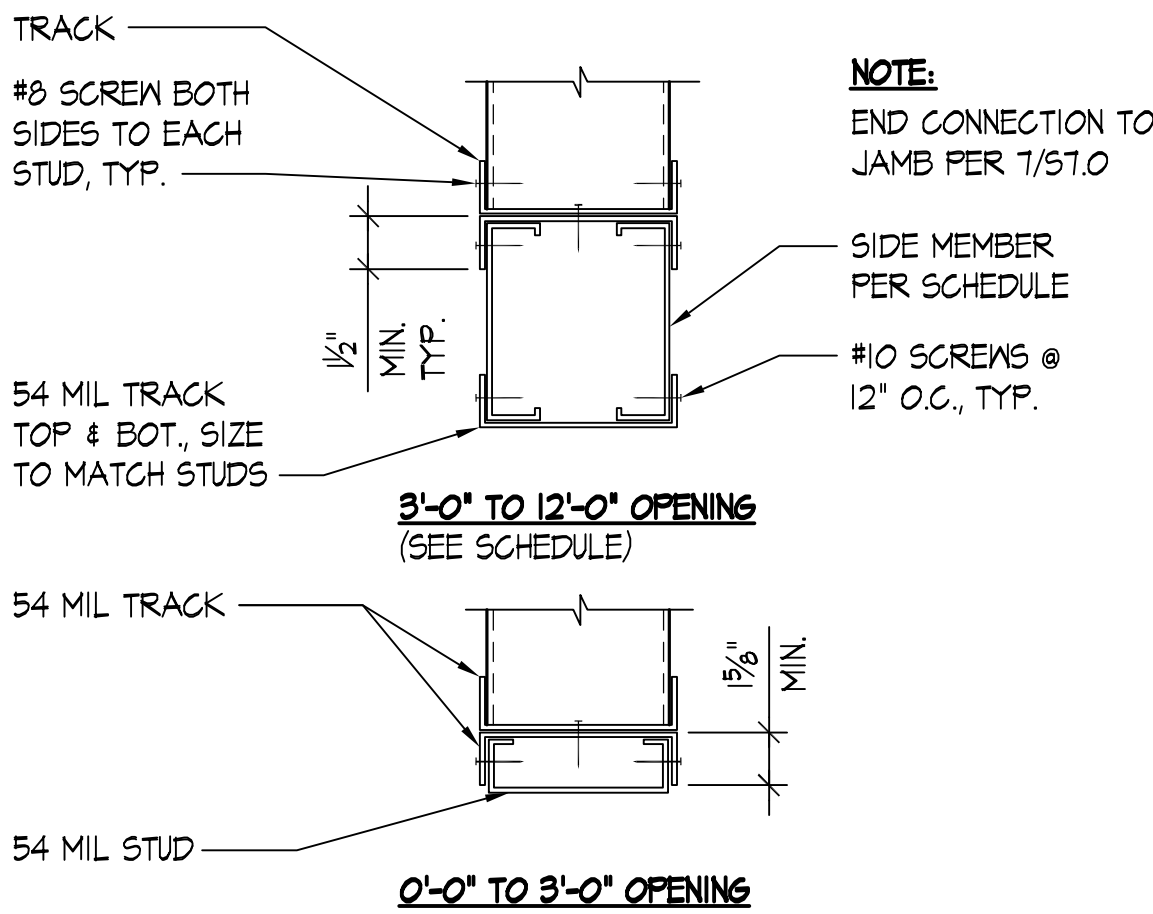
SCALE: NONE

TYPICAL HORIZONTAL STRAPPINGS/BLOCKING

SCALE: NONE

HEADER SCHEDULE

OPENING	SIDE MEMBER
< 8'	800S162-54
< 12'	1200S162-54



DETAIL

SCALE: NONE

TYPICAL HEADER

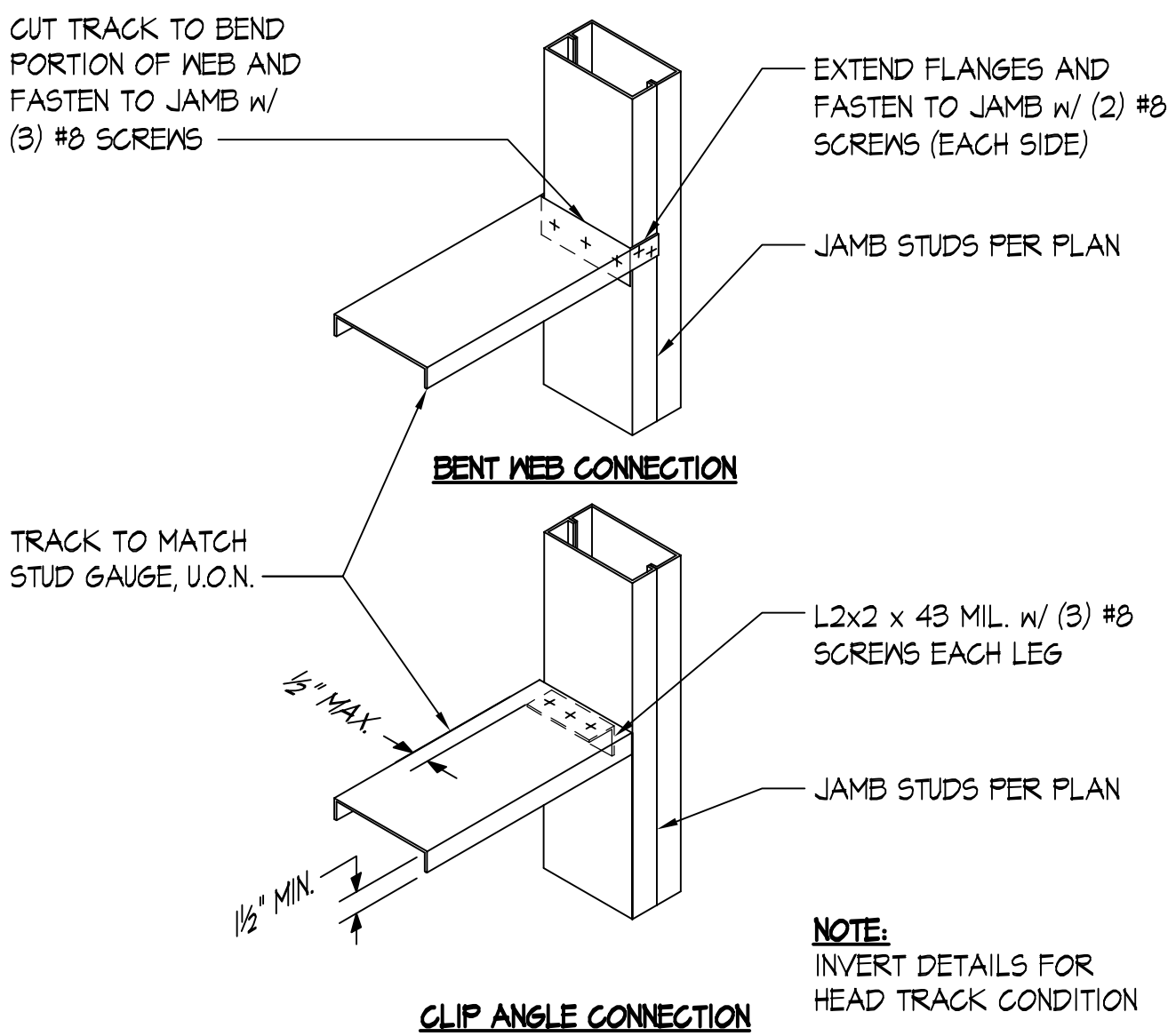
SCALE: NONE

TYPICAL HEADER CONNECTION

SCALE: NONE

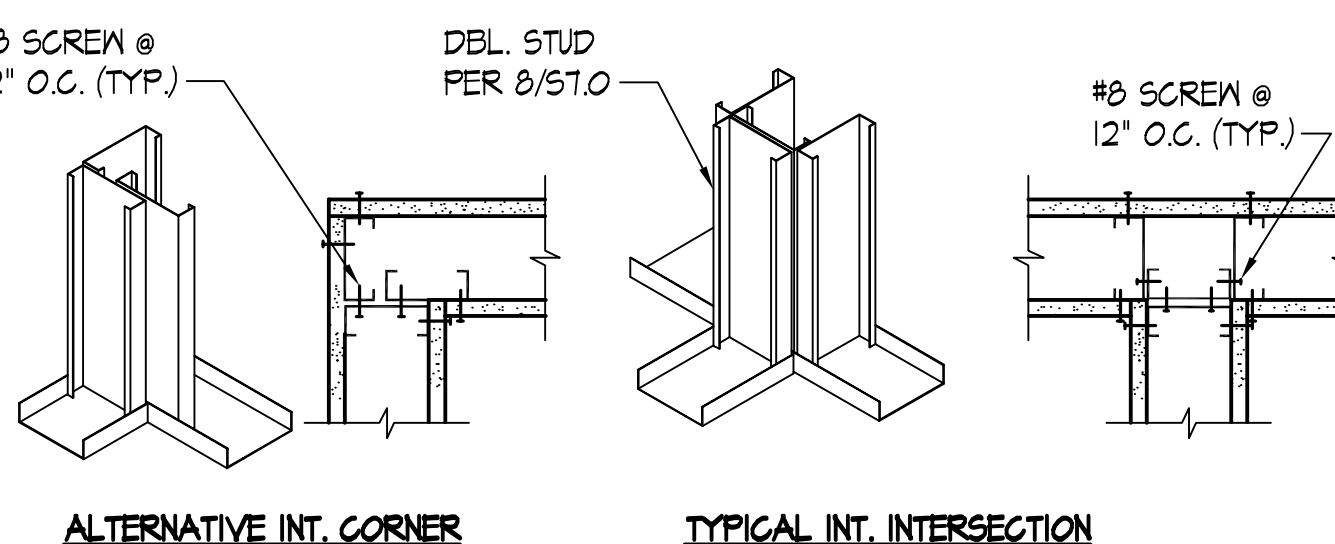
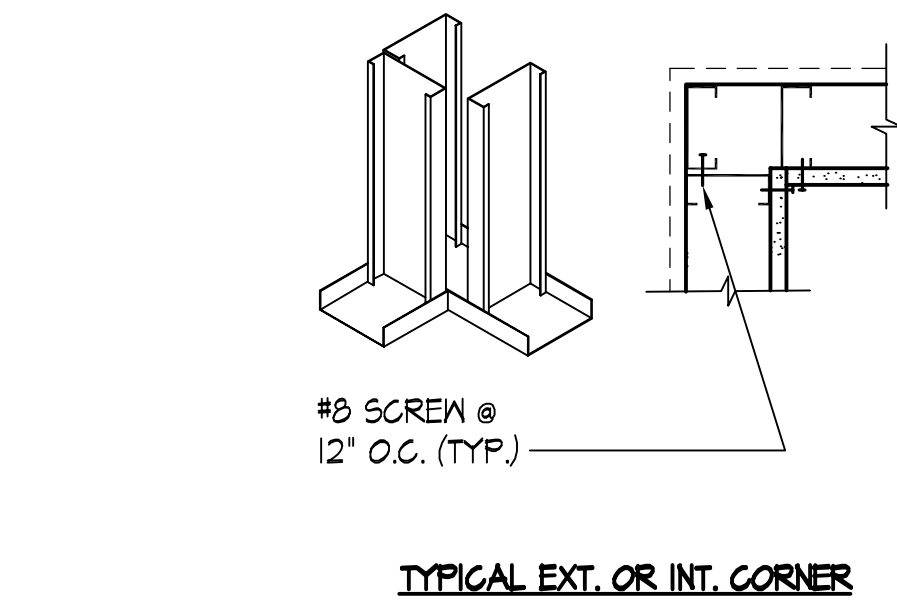
TYPICAL DOUBLE STUD

SCALE: NONE



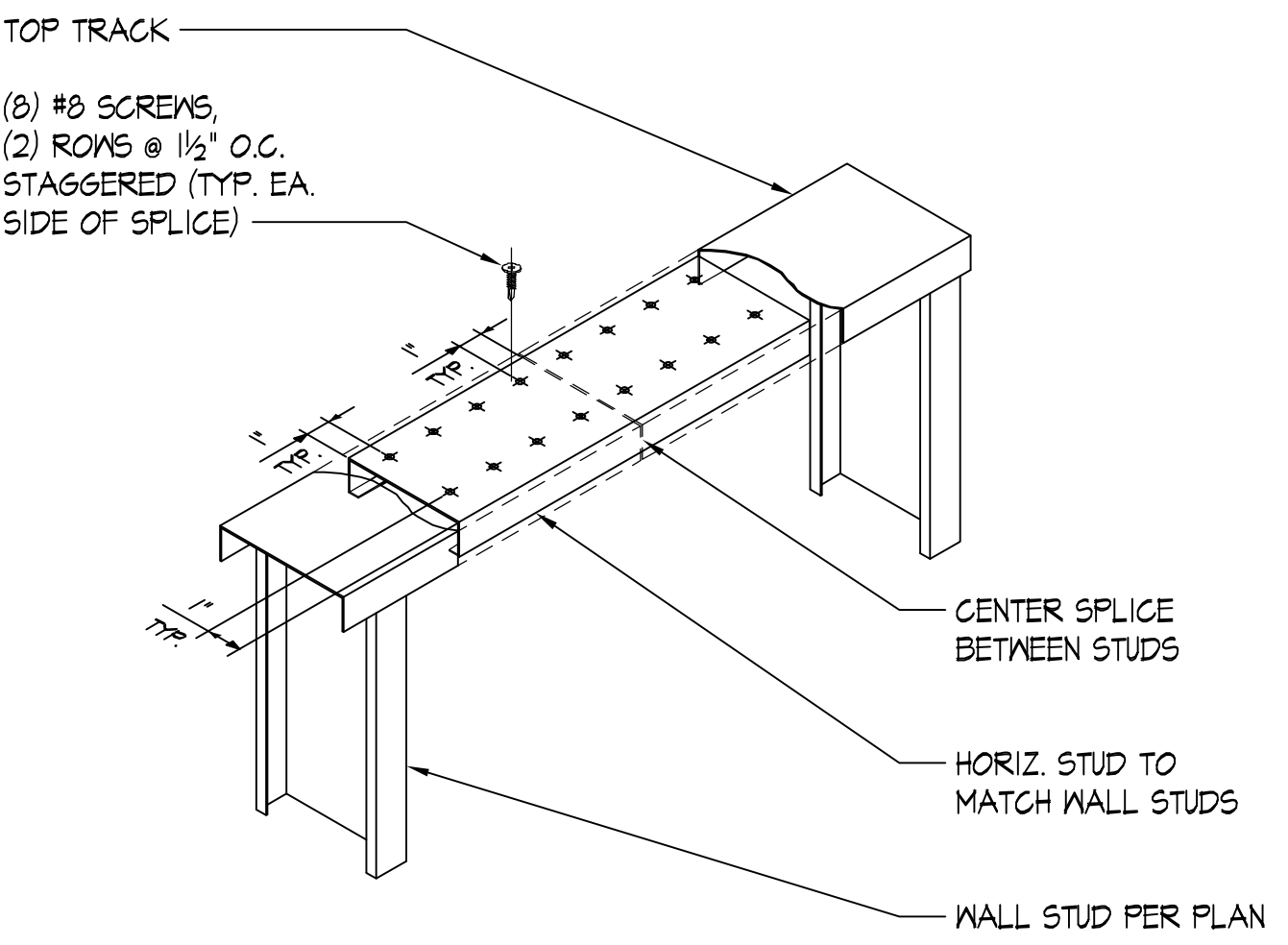
TYPICAL SILL AND HEAD TRACK CONNECTIONS

SCALE: NONE



TYPICAL WALL INTERSECTION

SCALE: NONE



TYPICAL TOP TRACK SPLICE

SCALE: NONE

TYPICAL LIGHT GAUGE METAL FASTENING SCHEDULE	
CONNECTION:	SHEET METAL SCREWS: ① ②
1. TOP & BOT. TRACK TO EA. STUD	#8 EA. SIDE
2. DOUBLE STUDS, FACE SCREW	(2) ROWS #8 @ 12" O.C.
3. TOP TRACK AT INTERSECTIONS, FACE SCREW	(4) #8
4. TOP TRACK AT LAPS, FACE SCREW	(6) #8
5. 1" x 8" SHEATHING OR LESS TO EACH BEARING, FACE SCREW	(2) #8
6. BUILT UP CORNER STUDS	#8 @ 12" O.C.
7. BUILT UP BEAMS	#8 @ 12" O.C. @ T&B, EA. SIDE

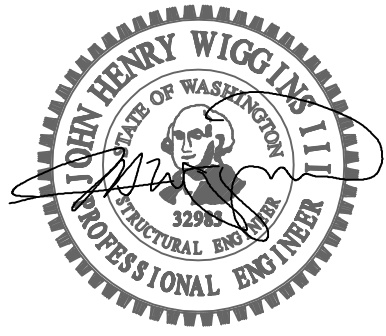
FOOTNOTES:

- U.O.N. ON PLAN OR OTHER DETAILS
- LARGER SCREWS MAY BE SUBSTITUTED FOR THOSE SHOWN

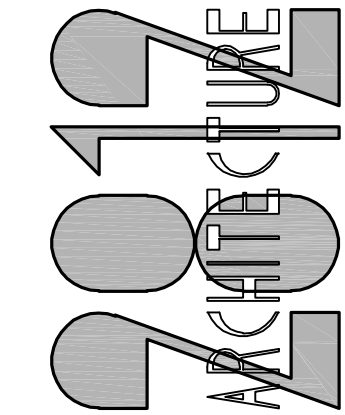
TYPICAL LIGHT GAUGE METAL FASTENING SCHEDULE

SCALE: NONE

For:	PERMIT SET	BID SET
Date:	4/22/20	7/22/20



2812 Colby Avenue  
Everett WA 98201  
(425) 252-2153 p  
www.2812architecture.com



Tenant Improvements for:  
**Stiliaguamish Tribal Courthouse**  
17014 58th Avenue NE  
Arlington, Washington

Contract: TYPICAL LIGHT GAUGE DETAILS

Drawing:	<b>S7.0</b>
Job Number:	20046.02







A. CONFIRM EXACT DEMOLITION SCOPE IN FIELD.

B. EXISTING DUCTWORK & DISTRIBUTION MAY NOT BE COMPLETELY ACCURATE. FIELD VERIFY WHERE IN SCOPE OF REMODEL.

— — — — — EXISTING TO REMAIN  
- - - - - DEMOLISH DUCTWORK & ASSOCIATED DIFFUSERS/GRILLES



MECHANICAL AND ELECTRICAL BUILDING DESIGN

111 AVE. C, SUITE 104  
SNOHOMISH, WA 98290

office: 360-863-6677  
fax: 360-863-3565

ARLINGTON, WASHINGTON 98223

[illegible]

2/2020

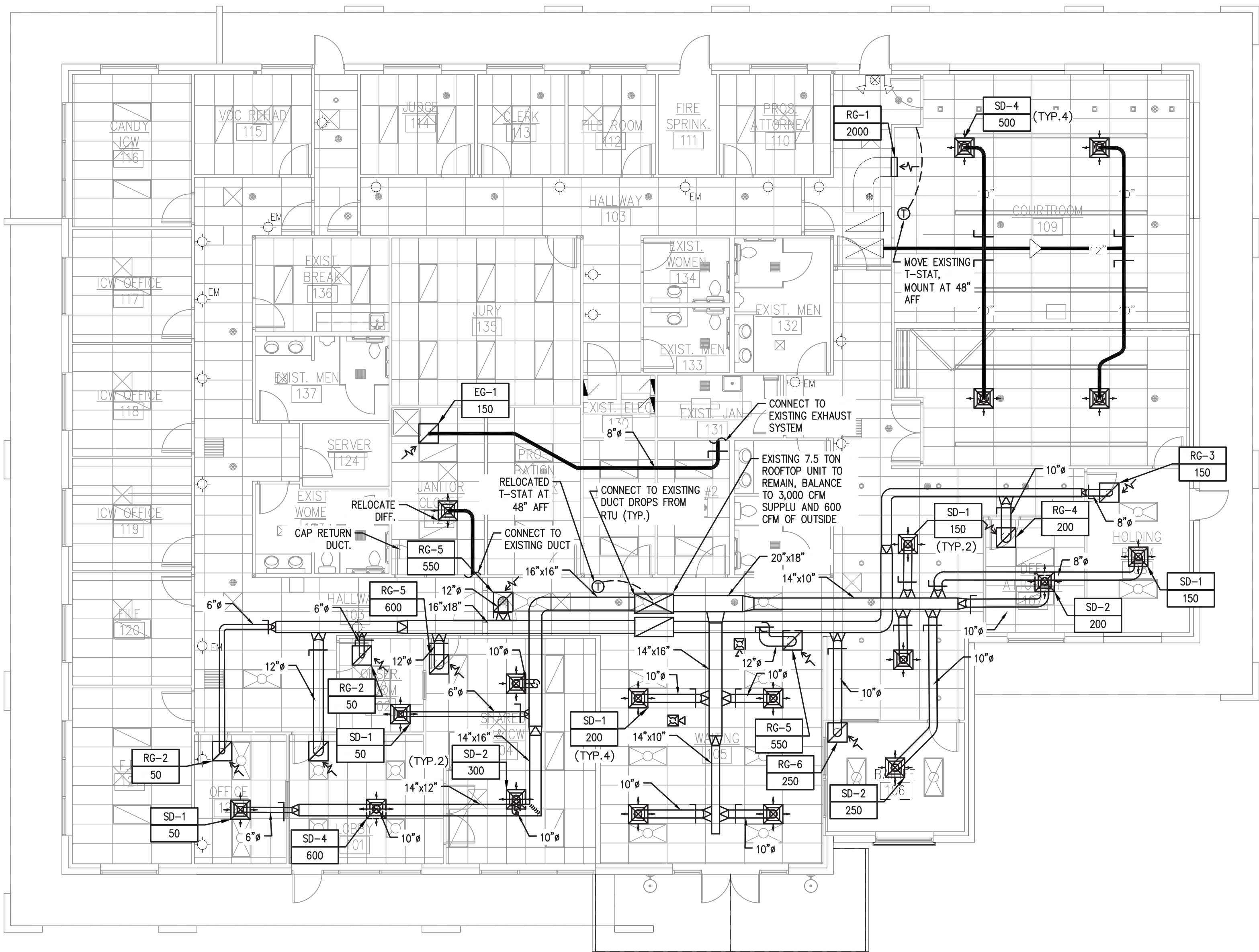
## MECHANICAL DEMO

AWN BY:	Salvador Campos
SIGNER:	Tucker Andrews
CHECKED BY:	Chris Rensch
SHEET	

M2.00



M2.01 MECHANICAL HVAC PLAN.rvt 7/22/2020 12:38 PM



## MECHANICAL HVAC PLAN

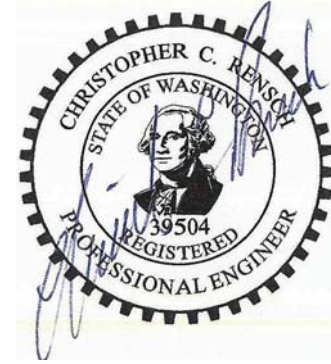
SCALE: 1/8" = 1'-0"



### GENERAL NOTES

- A. ALL IS NEW UNLESS NOTED OTHERWISE.
- B. INSULATE DUCTWORK PER CODE AND SPEC REQUIREMENTS (ALL NOT SHOWN). COORD. CLEARANCE REQUIREMENTS FOR ADDED INSULATION.
- C. SEE ARCH. SHEETS FOR PAINTING AND COLORS FOR ALL EXPOSED DUCT WORK, DIFFUSERS, GRILLES, ETC.

SEAL



### PROJECT TENANT IMPROVEMENTS FOR: STILLAGUAMISH TRIBAL COURTHOUSE

17014 59th Avenue NE  
ARLINGTON, WASHINGTON 98223

REVISIONS


DATE

07/22/2020

SHEET TITLE

MECHANICAL HVAC  
PLAN

### BID SET

DRAWN BY: Salvador Campos  
DESIGNER: Tucker Andrews  
CHECKED BY: Chris Rensch  
SHEET

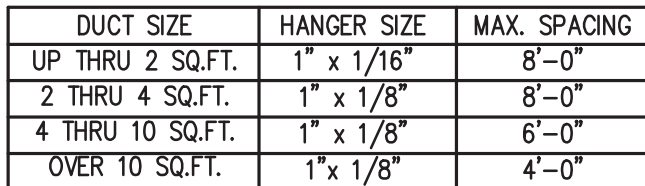
M2.01

**RENSCH ENGINEERING**

MECHANICAL AND ELECTRICAL BUILDING DESIGN  
111 AVE. C, SUITE 104  
SNOHOMISH, WA 98290  
office: 360-863-6677  
fax: 360-863-3565



Scale: NTS



## 2 DUCT HANGING DETAIL

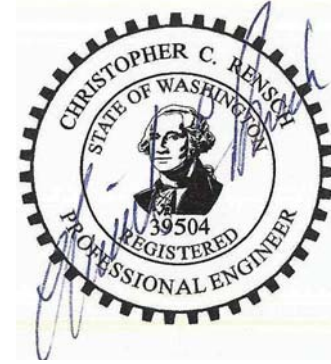
Scale: NTS

# RENSCH

MECHANICAL AND ELECTRICAL BUILDING DESIGN

111 AVE. C, SUITE 104  
SNOHOMISH, WA 98290

office: 360-863-6677  
fax: 360-863-3565



PROJECT  
TENANT IMPROVEMENTS FOR:  
STILLAGUAMISH TRIBAL COURTHOUSE

17014 35th AVENUE NE  
ARLINGTON, WASHINGTON 98223

PROJECT

## REVISIONS

DATE \_\_\_\_\_

07/22/2020

SHEET TITLE

## MECHANICAL DETAILS

## BID SET

DRAWN BY: Salvador Campos  
DESIGNER: Tucker Andrews  
CHECKED BY: Chris Rensch  
SHEET

# M5.01



MECH-01-MECHANICAL SCHEDULES.DWG 7/22/2020 12:38 PM

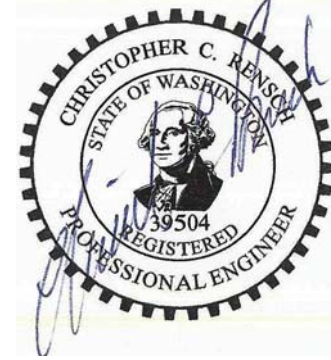
VENTILATION AIR CALCULATION (BASED ON THE 2015 IMC WITH STATE AMENDMENTS AND ASHRAE STANDARD 62.1)

MARK	AREA SERVED	OCCUPANCY CATEGORY	ZONE AREA (Az)	OCCUPANCY DENSITY (P/1000 SF)	ZONE POPULATION (Pz)	OUTDOOR AIR CFM PER PERSON (Rp)	OUTDOOR AIR CFM PER SF (Ra)	ZONE AIR EFFECTIVNESS (Ez)	CALCULATED OUTDOOR AIR (CFM)		% REQUIRED OUTDOOR AIR SUPPLIED TO AREA
										MINIMUM [CFM]	
RTU-5	LOBBY (101)	OFFICE - MAIN ENTRY LOBBIES	215	10	3	5	0.06	0.8	35	40	115%
	OBSERVATION ROOM (102)	OFFICE - OFFICE SPACE	85	5	1	5	0.06	0.8	13	15	119%
	SHARED FR&IW (104)	EDUCATION - DAYCARE	385	25	10	10	0.18	0.8	212	215	102%
	WAITING (105)	OFFICE - RECEPTION AREA	550	30	17	5	0.06	0.8	148	150	102%
	HALLWAY (105)	OFFICE - OFFICE SPACE	800	0	0	0	0.06	0.8	60	65	109%
	BAILEF (106)	OFFICE - OFFICE SPACE	170	5	1	5	0.06	0.8	19	20	106%
	ATTORNEY (107)	OFFICE - OFFICE SPACE	120	5	1	5	0.06	0.8	15	15	99%
	HOLDING (108)	CORRECTIONAL FACILITIES -CELLS	185	25	5	5	0.12	0.8	59	60	102%
	OFFICE (122)	OFFICE - OFFICE SPACE	130	5	1	5	0.06	0.8	16	20	125%
TOTAL									576	600	105%

1. HVAC UNITS SHALL BE BALANCED TO PROVIDE THE SELECTED OUTDOOR AIR QUANTITIES INDICATED.

GRILLES & DIFFUSERS SCHEDULE (1)							
TAG	DESCRIPTION	MOUNTING	NECK SIZE [IN.]	FACE SIZE [IN.]	CFM RANGE	MANUFACTURER & MODEL	NOTES
SD-1	SUPPLY DIFFUSER	LAY-IN	6Ø	24x24	50-200	TITUS TMS	(2)
SD-2	SUPPLY DIFFUSER	LAY-IN	8Ø	24x24	201-300	TITUS TMS	(2)
SD-3	SUPPLY DIFFUSER	LAY-IN	10Ø	24x24	301-425	TITUS TMS	(2)
SD-4	SUPPLY DIFFUSER	LAY-IN	12Ø	24x24	426-600	TITUS TMS	(2)
RG-1	RETURN GRILLE	SURFACE	24x24	26x26	2000	TITUS 350RL	(2)
RG-2	RETURN GRILLE	LAY-IN	6Ø	24x24	50-1500	TITUS 350RL	(2)
RG-3	RETURN GRILLE	SURFACE	8Ø	24x24	50-1500	TITUS 350RL	(2)
RG-4	RETURN GRILLE	LAY-IN	10Ø	24x24	50-1500	TITUS 350RL	(2)
RG-5	RETURN GRILLE	LAY-IN	12Ø	24x24	50-1500	TITUS 350RL	(2)
RG-6	RETURN GRILLE	SURFACE	10Ø	24x24	50-1500	TITUS 350RL	(2)
EG-1	EXHAUST GRILLE	LAY-IN	8Ø	24x24	175	TITUS 350RL	(2)

- NOTES:
- SEE PLANS FOR QUANTITIES.
  - CONFIRM FINISH WITH ARCHITECT AND OWNER.



PROJECT  
TENANT IMPROVEMENTS FOR:  
**STILLAGUAMISH TRIBAL COURTHOUSE**  
17014 59th Avenue NE  
ARLINGTON, WASHINGTON 98223

REVISIONS

DATE	07/22/2020
SHEET TITLE	MECHANICAL SCHEDULES
BID SET	
DRAWN BY:	
DESIGNER:	
CHECKED BY:	
SHEET	

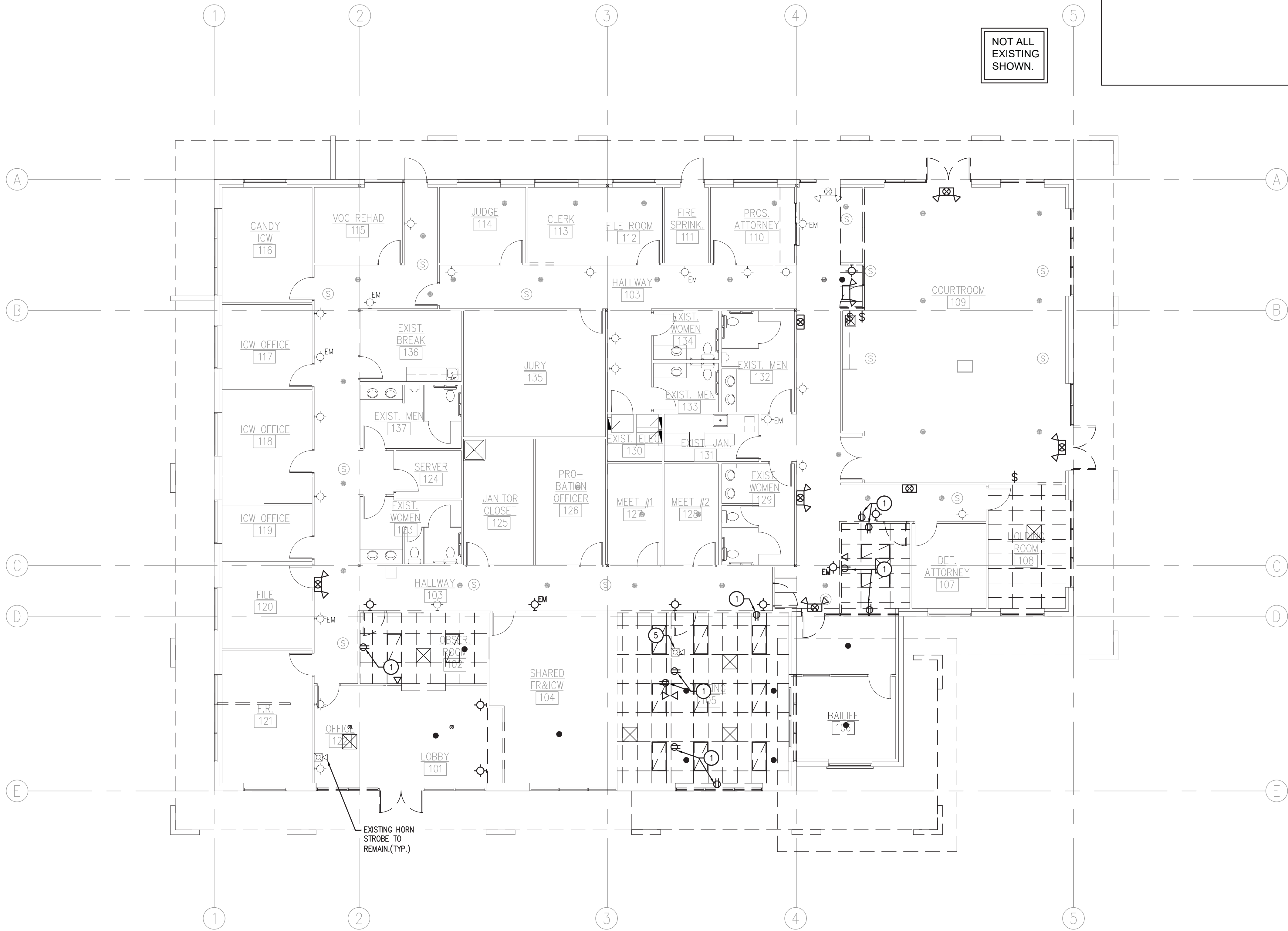
M6.01







E2.00 ELECTRICAL DEMO PLAN.dwg 7/22/2020 10:37 AM



**ELECTRICAL DEMO PLAN**  
SCALE: 1/8" = 1'-0"

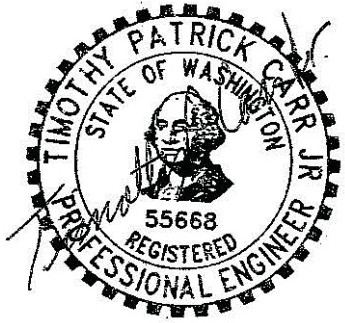
## GENERAL NOTES

- DEMO ITEMS SHOWN AS DASHED.
- KEEP DEMO'D ITEMS FOR OWNER.
- REROUTE CIRCUITING FOR EXISTING WIRES INSIDE WALLS OR AREAS TO BE DEMO'D.
- ITEMS TO BE DEMO'D ARE SHOWN DARK. EXISTING IS SHOWN FADED.

## #

## KEY NOTES

- DEMO RECEPTACLE'S CIRCUIT SHALL BE RELOCATED AND EXTENDED TO NEW A LOCATION FOR NEW OUTLET. REFER TO POWER PLAN.
- CONFIRM IF EXISTING CONVENIENCE RECEPTACLE IS LOCATED AS SHOWN & PROVIDE NEW ONE IF NOT. (COORD. W/ OWNER.)
- WALL SCONCE: EXISTING FIXTURE TO BE REUSED/RELOCATED. INSPECT & FIELD VERIFY IF FIXTURE IS IN GOOD CONDITION AND REPLACE LAMPING AS NEEDED. PATCH & PAINT DRYWALL TO MATCH EXISTING AS NEEDED.
- REMOVE LIGHT FIXTURE AND PATCH/CAP EXISTING CEILING AS NEEDED.
- REMOVE AND RELOCATE HORN/STROBE. SEE POWER PLAN FOR NEW LOCATION.



## PROJECT TENANT IMPROVEMENTS FOR: STILLAGUAMISH TRIBAL COURTHOUSE

17014 59th Avenue NE  
ARLINGTON, WASHINGTON 98223

## REVISIONS

NO.	DESCRIPTION	DATE

## DATE

07/22/2020

## SHEET TITLE

ELECTRICAL DEMO  
PLAN

## BID SET

DRAWN BY: Adrian Caple  
DESIGNER: Josh Ennsley  
CHECKED BY: Tim Carr  
SHEET

E2.00

SEAL

**RENSCH ENGINEERING**

MECHANICAL AND ELECTRICAL BUILDING DESIGN

office: 360-863-6677  
111 AVE. C. SUITE 104  
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GENERAL NOTES	
1.	CONFIRM ALL FIXTURE TYPES AND FINISHES WITH ARCHITECT AND ENSURE FIXTURES INSTALLED DO NOT EXCEED INPUT WATTS INDICATED.
2.	ALL FINISH/COLOR BY OTHERS
3.	EC SHALL BE RESPONSIBLE TO INSTALL ALL CONTROL COMPONENTS AND WIRING PER MANUF'S REQUIREMENTS.
4.	PROVIDE CONNECTIONS AND ACCESSORIES AS NEEDED.


[illegible]

**ELECTRICAL LIGHTING PLAN**  
 SCALE: 1/8" = 1'-0"  
 0 4' 8' 16'

Notes:

- 1 Provide Night Light control for fixtures indicated.
- 2 30-minute delay off delay on Automatic Control.
- 3 Daylight zones defined by WSEC.
- 4 Manual control for area where automatic OFF may endanger occupant safety or for security.
- 5 High security area shall have lights ON during occupied times with no manual OFF control in the room.

<p><b><u>SWITCHING CIRCUIT LEGEND</u></b></p>	
<p>NOTE: CONFIRM ACTUAL SWITCH LEG REQUIREMENTS INCLUDING ANY LOW VOLTAGE OR WIRELESS CONTROL REQUIREMENTS.</p>	
	TIMECLOCK SWITCH LEG(S)
	OCCUPANCY SENSOR SWITCH LEG

	PHOTO SENSOR SWITCH LEG
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PROJECT  
TENANT IMPROVEMENTS FOR:  
**STILLAGUAMISH TRIBAL C**  
17014 59th Avenue NE  
CARLINGTON, WASHINGTON 98223

**DATE**

07/22/2020

**SHEET TITLE**

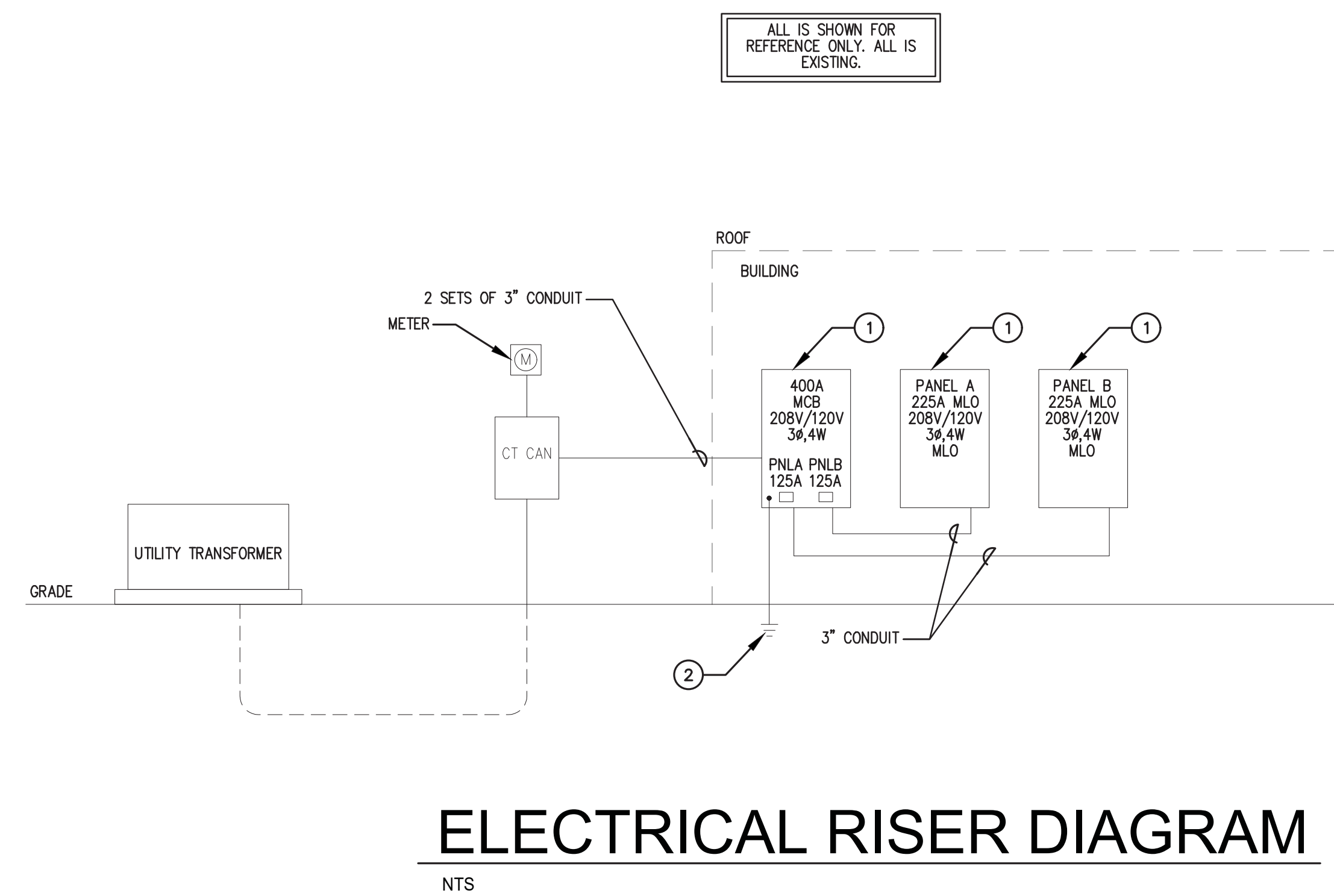
SHEET

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# E3.01



GENERAL NOTES	KEY NOTES
<p>A. ALL IS EXISTING UNLESS OTHERWISE NOTED.</p> <p>B. ALL PANELS/EQUIPMENT SHALL BE PERMANENTLY LABELED FOR "ARC FLASH HAZARD" AS REQUIRED BY SEC. 110.16 OF THE NEC.</p> <p>C. PANELS ARE EXISTING AND NOT BEING CHANGED. ANY BREAKERS, IF REPLACED, SHALL HAVE THE SAME AIC RATING AS OTHER BREAKERS IN THE PANEL.</p> <p>D. LOADS ON THE PANELS ARE NOT BEING INCREASED. CIRCUITS OF DEMO'D ITEMS ARE BEING RELOCATED.</p>	<p>1. SEE PANEL SCHEDULE FOR CIRCUIT DIRECTORY.</p> <p>2. VERIFY GROUNDING SYSTEM INSTALLED PER CODE.</p>



NTS

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STILLAGUAMISH TRIBAL COURTHOUSE  
17014 59th Avenue NE

ARLINGTON, WASHINGTON 98223

[illegible]

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20

**TITLE**

## CRITICAL RISER GRAMS

BID SET

BY:	Adrian Caple
ER:	Josh Ensley
ED BY:	Tim Carr

## 15.01



NOTE: PANEL, BREAKERS AND LOADS ARE ALL EXISTING UNLESS NOTED OTHERWISE. IF GFI OR REPLACEMENT BREAKERS ARE NOT AVAILABLE, F&I NEW PANEL.

EXISTING PANEL PANEL A (EXISTING)																
VOLTAGE: 208V / 120V 3 Ph/4 W FED FROM: MDP BUS RATING: 225 AMP HIGH LEG? No					MOUNTING OPTIONS					MAINS DEVICE MLO MAIN OCPD RATING: 125 AMPS AIC RATING: SEE RISER LOCATION: EXISTING ELEC ROOM FEED THRU LUGS No						
# OF SPACES 42																
NOTES	SERVES	TYPE LOAD	CKT BRKR AMP	POLE	CKT NO.	CKT LOAD	PHASE LOADS			CKT LOAD	CKT NO.	CKT BRKR AMP	POLE	TYPE LOAD	SERVES	NOTES
	LIGHTING EXTERIOR		20 /1		1		0				2	20 /1			LIGHTING	
	LIGHTING EXTERIOR		20 /1		3			0			4	20 /1			LIGHTING	
	LIGHTING EXTERIOR		20 /1		5				0		6	20 /1			LIGHTING	
	LIGHTING SIGN		20 /1		7		0				8	20 /1			LIGHTING	
	LIGHTING POLE		20 /1		9			0			10	20 /1			LIGHTING	
	LIGHTING POLE		20 /1		11				0		12	20 /1			SPARE	
	CORRIDORS		20 /1		13		0				14	20 /1			FISH TANKS	
	FIRE ALARM		15 /1		15			0			16	20 /1			ROOF RECEPTACLE	
NEW	METAL DETECTOR	NC	20 /1		17	20			20		18					
					19		0				20					
					21			0			22					
					23				0		24					
					25		0				26					
					27			0			28					
					29				0		30					
					31		0				32					
					33			0			34					
					35				0		36					
					37		0				38					
					39			0			40					
					41				0		42					

NOTES:

\* IF RECEPTACLE LOAD IS OVER 10KVA THEN THE AMOUNT OVER 10KVA IS COUNTED AS 50% FOR NON-DWELLING UNITS PER NEC 220.44.

NEW- NEW BREAKERS. NEW CIRCUIT.

ADD NEW 15 AMP  
BREAKER AT CIRCUIT  
NUMBER 17 FOR NEW  
METAL DETECTOR

NOTE: PANEL, BREAKERS AND LOADS ARE ALL EXISTING UNLESS NOTED OTHERWISE. IF GFI OR REPLACEMENT BREAKERS ARE NOT AVAILABLE, F&I NEW PANEL.

EXISTING PANEL MDP (EXISTING)																			
VOLTAGE: FED FROM: BUS RATING: HIGH LEG?					208V / 120V    3 Ph/4 W CT CAN 400 AMP No					MOUNTING OPTIONS			SURFACE		MAINS DEVICE MAIN OCPD RATING: AIC RATING: LOCATION: FEED THRU LUGS			MCB 400 AMPS SEE RISER EXISTING ELEC ROOM No	
# OF SPACES					24														
NOTES	SERVES		TYPE LOAD	CKT BRKR AMP    POLE	CKT NO.	CKT LOAD	PHASE LOADS			CKT LOAD	CKT NO.	CKT BRKR AMP    POLE	TYPE LOAD	SERVES				NOTES	
	SPACE				1		0				2	40 /3		RTU-3					
					3			0			4								
					5				0		6								
	RTU-5			40 /3	7		0				8	35 /3		RTU-4					
					9				0		10								
					11					0	12								
	CV-1			20 /2	13		0				14	20 /2		RTU-1					
					15				0		16								
	RTU-2			20 /2	17					0	18			SPACE					
					19				0		20								
					21						22								
					23					0	24								

NOTES:

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LOADS IN THIS PROJECT ARE NOT BEING INCREASED, ONLY  
MOVED AROUND IN THE SPACE. LIGHTING IS BEING  
REDUCED BY REPLACING FLOURESCENT WITH LED.

NOTE: PANEL, BREAKERS AND LOADS ARE ALL EXISTING UNLESS NOTED OTHERWISE. IF GFI OR REPLACEMENT BREAKERS ARE NOT AVAILABLE, F&I NEW PANEL.

EXISTING PANEL PANEL B (EXISTING)														
VOLTAGE: 208V / 120V 3 Ph/4 W FED FROM: MDP BUS RATING: 225 AMP HIGH LEG? No					MOUNTING OPTIONS SURFACE   # OF SPACES 42					MAINS DEVICE MLO MAIN OCPD RATING: 125 AMPS AIC RATING: SEE RISER LOCATION: EXISTING ELEC ROOM FEED THRU LUGS No				
NOTES	SERVES	TYPE LOAD	CKT BRKR AMP POLE	CKT NO.	CKT LOAD	PHASE LOADS			CKT LOAD	CKT NO.	CKT BRKR AMP POLE	TYPE LOAD	SERVES	NOTES
	RECEPTACLES		20 /1 1	1		0				2	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 1	3			0			4	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 5	5				0		6	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 7	7		0				8	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 9	9			0			10	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 11	11				0		12	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 13	13		0				14	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 15	15			0			16	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 17	17				0		18	20 /1		RECEPTACLES	
	RECEPTACLES		20 /1 19	19		0				20	20 /1		RECEPTACLES	
	EF-1		20 /1 21	21			0			22	20 /1		RECEPTACLES	
	EH-1		20 /1 23	23				0		24	20 /1		WH-1 BLOWER	
	HEATER		20 /1 25	25		0				26	20 /1		HEAT TRACE	
	COFFEE MAKER - RM 116		20 /1 27	27			0			28	20 /1		COPIER	
	MICROWAVE - RM 116		20 /1 29	29				0		30				
	REFRIGERATOR - RM 116		20 /1 31	31		0				32				
				33			0			34				
				35				0		36				
				37		0				38				
				39				0		40				
				41				0		42				

NOTES:

\* IF RECEPTACLE LOAD IS OVER 10KVA THEN THE AMOUNT OVER 10KVA IS COUNTED AS 50% FOR NON-DWELLING UNITS PER NEC 220.44.



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REVISIONS

DATE  
07/22/2020  
SHEET TITLE

PANEL SCHEDULES

BID SET

DRAWN BY: Adrian Caple  
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CHECKED BY: Tim Carr  
SHEET

E6.01

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